ADDENDUM NO. 1
ISSUE DATE: August 22, 2019

RE: **BASEBALL FIELD RENOVATIONS**
UNIVERSITY OF WISCONSIN – STEVENS POINT
STEVENS POINT, WISCONSIN
UWSA Project No. **K-19-001**

BID OPENING: GPC Bidders: **2:00 P.M., August 29, 2019**

FROM: **SmithGroup**
44 East Mifflin Street, Suite 500
Madison, Wisconsin 53703

TO: Prospective Bidders

This addendum forms a part of the Bid Documents and modifies the original Bid Documents dated July 30, 2019 as noted below. Acknowledge receipt of this Addendum by inserting the number and issue date of this addendum in the blank space provided on the Bid Form. Failure to do so may subject the Bidder to disqualification.

This Addendum consists of (1) one page and the attached documents, S**pecification Section 02 32 00 – Geotechnical Investigation, and full-size Drawings CS100, CS101, CS102 and CS500**.

CHANGES TO SECTION 11 68 33 – ATHLETIC EQUIPMENT

1. Page 3, line 48, ADD: "Where Hot Dipped Galvanized Assembly Hardware is identified, use of Zinc-plated hardware will also be acceptable."

2. Page 4, line 6, ADD: "2. Top: 2' x 28' Text "ZIMMERMAN FIELD"

CHANGES TO DRAWINGS:

3. Drawing CS100 – OVERALL SITE PLAN: Delete Drawing CS100 and replace with revised CS100, attached.

4. Drawing CS101 – DEMOLITION AND EROSION CONTROL PLAN: Delete Drawing CS101 and replace with revised CS101, attached.

5. Drawing CS102 – LAYOUT PLAN ENLARGEMENT AND DETAILS: Delete Drawing CS102 and replace with revised CS102, attached.

6. Drawing CS500 – SITE DETAILS – SITE PREPARATION: Delete Drawing CS500 and replace with revised CS500, attached.

END OF ADDENDUM NO. 1

SECTION 02 32 00
GEOTECHNICAL INVESTIGATION
BASED ON DFD MASTER SPECIFICATION DATED 11/21/13

PART 1 - GENERAL

SCOPE

This section provides information resulting from subsurface investigations completed at the site as part of this project. This section may contain information applicable to ALL sitework, and other technical specification sections, as well. All Contractors are expected to review this information as part of their duties to familiarize themselves with the site.

Results of the geotechnical investigation apply only to the locations at which data was collected, at the specific time it was collected. Geotechnical conditions may differ elsewhere on the site.

Prior to making additional investigations of his own using test pits, borings, or other methods; Bidder shall first gain permission from property owner and UWSA Project Manager. Geotechnical investigations completed by Bidder shall comply with all applicable requirements of Division 01 through Division 33 of this project.

RELATED WORK

Applicable provisions of Division 01 govern work under this Section.

03 30 00 – Cast in Place Concrete
11 68 33 – Athletic Equipment

PART 2 - MATERIALS

Not used.

PART 3 - EXECUTION

Not used.

END OF SECTION

NTS NUMMELIN TESTING SERVICES, INC.

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SUBSURFACE SOIL INVESTIGATION

PROPOSED UWSP RESIDENCE HALL
ILLINOIS AVENUE
STEVENS POINT
WISCONSIN

NTS 141.04

C08036

PREPARED FOR:

ENGBERG ANDERSON
1 NORTH PINCKNEY STREET
P.O. BOX 100
MADISON, WI 53703-0100

ATTENTION: MR. JAMES BROWN

FIELD INVESTIGATION BY:

NUMMELIN TESTING SERVICES, INC.
3217 WHITING AVENUE
STEVENS POINT, WI

JULY 16, 2008

SUBSURFACE SOIL INVESTIGATION

PROPOSED UWSP RESIDENCE HALL
ILLINOIS AVENUE
STEVENS POINT
WISCONSIN

1. INTRODUCTION

Nummelin Testing Services, Inc. (NTS) performed this investigation for the purpose of providing preliminary site information for the proposed University of Wisconsin – Stevens Point (UWSP) Residence Hall to be built on Illinois Avenue in the City of Stevens Point, Portage County, Wisconsin. The results and recommendations reported are based upon information obtained during a field investigation with borings, and the geotechnical analysis of that information. This report should be considered in its entirety during design and construction; no one section of the report should be used without considering all other sections.

The conclusions and recommendations reported are based on our interpretation of available subsurface and project information. The report may not represent variations that occur between or away from boring locations.

Should the scope of this project be altered, or if subsurface variations become evident during construction, it may be necessary to modify our recommendations. See the attached Geotechnical Engineering Report Information sheet for general information on NTS's geotechnical reports.

2. PROJECT DESCRIPTION

The proposed project is the construction of a 4-story, dormitory-type, load-bearing masonry building with full basement on Illinois Avenue. The maximum interior wall bearing load is estimated at 30 kips, and the maximum exterior wall bearing load is estimated at 15 kips per lineal foot. The first floor elevation is expected to be near the existing grade.

At the time of the investigation, the surface of the site was covered by four tennis courts, three volleyball courts, and several clusters of trees. It is our understanding that this is a preliminary investigation, and another investigation will be performed at a later date.

3. FIELD INVESTIGATION

Four (4) standard penetration soil borings were performed on July 7, 2008, with a truck-mounted, rotary drilling rig at locations shown on the attached sketch. Engberg Anderson determined the proposed boring locations and depths. At the time of drilling, NTS located the borings in the field. Boring 1 was drilled 18 feet south of the proposed location, and Boring 2 was drilled 8 feet west of the proposed location because trees blocked drill rig access to the proposed locations of Borings 1 and 2. Borings 3 and 4 were drilled at the proposed locations. Drilling between samples was by the hollow-stem-auger technique. Borings 1 and 2 were terminated at the proposed depth of 15 feet. Borings 3 and 4 were proposed to be drilled to a depth of 50 feet but were terminated because of auger refusal at depths of 32 and 19.5 feet, respectively.

Representative soil samples were obtained while boring using the Standard Penetration Test (SPT) method, in general accordance with ASTM Test Procedure D1586, at the depths indicated on the boring logs. The soils were visually classified in general accordance with ASTM Test Procedure D2488 by a technician at the time the borings were performed. The technician transferred the SPT samples to clean, glass jars immediately after retrieval and transported the samples to the NTS lab after completion of field operations. The soil samples have been examined in the laboratory by this writer and classified according to the USCS and, as requested, the USDA soil classification system. Both the USCS and USDA classifications appear on the logs. Soil parameters and classifications reported herein have been estimated based on the standard penetration test results, soil descriptions, and experience with soils in this area. No lab testing has been performed.

After completion of the borings, the bore holes were backfilled with bentonite chips to comply with WDNR requirements, and the last few inches were patched with auger cuttings.

At the time of the investigation, NTS determined ground elevations at the boring locations. The top of the large spout of the fire hydrant at the southeast corner of the site was used as a benchmark. According to the City of Stevens Point Engineering Department, the elevation of this benchmark is 1091.84.

Copies of the soil boring logs and a location sketch are appended to this report.

4. SUBSURFACE CONDITIONS

4. 1. Area Geology

The subsoils in this area are mapped as outwash deposits. Outwash deposits generally consist of stratified sand and/or stratified sand and gravel. The underlying rock is mapped as granitic intrusive rock that is present at depths of less than 50 feet below the average surface terrain. A soil survey of Portage County maps the near-surface soils in this area as Roscommon muck that has been cut and filled at various locations. According to old soil maps, it appears that there used to be a creek near this area.

Note that mapped soil and bedrock conditions are provided for additional information only. Use of a design based on mapped or assumed conditions is not recommended.

4. 2. Soils at the Boring Locations

The general soil profile encountered at this site was silty sand fill and/or sedimentary peat from the surface to depths of 2 to 7 feet over very loose to medium-dense, native sand to depths of 13 to 17 feet over stiff clay (probably weathered from the underlying granite) to depths of 17 to 26.5 feet over granite bedrock, weathered to various degrees. The degree of weathering of the bedrock decreased with depth. A summary of the soil profile encountered in each boring is presented in Table 1.

Table 1. Soil Profiles in the Borings.

Boring #	Elevation	Sand Fill	Peat	Native Sand	Clay	Bedrock
1	1089.47	0' – 3'	–	3' – 14'	14' – 15'+	–
2	1090.92	0' – 3'	–	3' – 15'+	–	–
3	1088.67	0' – 6"	6" – 2'	2' – 17'	17' – 26.5'	26.5' – 32'+
4	1091.47	0' – 3'	3' – 7'	7' – 13'	13' – 17'	17' – 19.5'+

See individual boring logs for more detailed soil descriptions.

4. 3. Water Level Measurements

Water was encountered in all borings at depths of 3 to 6 feet (elevation 1085.12 to 1085.67). These water level measurements should be considered as representative of site conditions at the time of boring only. Expect seasonal fluctuations in the groundwater table of up to two feet.

5. DISCUSSION AND RECOMMENDATIONS

5. 1. General

Plan to completely remove all existing pavement, any existing foundations, topsoil, and trees from the building footprint prior to further excavation and site grading.

The soils at this site, with no modifications, are expected to provide variable support for shallow foundations. Peat and fill occurred in the borings to depths of 2 to 7 feet, and, based on soil mapping of the area, the peat and fill likely occur elsewhere at this site, possibly to deeper depths. Undercutting to remove peat and/or fill from the base of shallow foundation excavations should be expected. The amount of undercutting will depend on the proposed depth of the foundations. The native sand encountered below the fill and peat was very loose to medium-dense. Compaction of the native sand will probably be necessary where the sand occurs at the base of excavations for shallow foundations.

Construction of a basement is not recommended at this site because, with water at depths of 3.0 to 6.0 feet (elevation 1085.12 to 1085.67), the potential for flooding of a below-ground level is high. Should a below-ground level be constructed, it should be constructed at the highest elevation possible with an extensive underdrain system to prevent flooding.

Expect dewatering to be necessary for excavations near or below elevation 1085.7 at this site.

See below for further recommendations.

5. 2. Foundations and Floor Slab Support

Standard spread footings may be used at this site, but be aware that undercutting and/or compacting of existing soils will probably be necessary for shallow foundations. Deep foundations may also be used, but further exploration of the bedrock profile should be considered for deep foundations at this site because the bedrock depth and degree of weathering varied between borings. The depth to bedrock and degree of weathering have also been found to vary significantly over short distances in other areas around Stevens Point. Intermediate foundations (rammed aggregate piers) may also be considered.

A review of construction plans and records, if available, for nearby buildings may be beneficial for determining the most economical foundation type.

It is recommended that all load-bearing soils be inspected by NTS or another qualified soils engineering firm to verify that adequate bearing is present.

If the recommendations in this report are followed, total settlement should not exceed 1 inch, and differential settlement should not exceed one-half inch.

5. 2. 1. Shallow Foundations

It is recommended that shallow foundations bear on the native sand or on fill meeting the requirements of section 5. 4. that bears on the native sand. Note that the native sand appeared very loose in places during boring, and undercutting/compaction of the native sand should be expected below shallow foundations. A presumptive allowable bearing capacity of 3,000 pounds per square foot (psf) may be used in the design of footings if the soil layer just below the footings is comprised of native sand and/or fill that meets these two requirements: the thickness of the layer must be at least one footing's width or 18 inches, whichever thickness is greater, and all soil in the layer must be compacted to at least 95% of the modified Proctor density (ASTM D-1557).

Shallow foundations should not bear on the peat or existing fill. Peat and/or fill were encountered in Borings 1 through 3 to depths of 2 to 3 feet and to a depth of 7 feet in Boring 4, and soil maps of the area suggest this peat and fill are likely to occur elsewhere at this site, possibly to deeper depths. Expect to undercut where peat and/or existing fill are found at the base of shallow foundation excavations.

5. 2. 2. Intermediate Foundations (Rammed Aggregate Piers)

NTS is not aware of rammed aggregate piers used for other buildings in Stevens Point, but sites with loose sands and high water table, such as this site, may be a good candidate for rammed aggregate piers. Rammed aggregate piers are constructed by ramming thin layers of aggregate into the ground with a high energy tamper. This forms a dense column of aggregate that offers support for bearing pressures of up to 10,000 psf. Rammed aggregate piers can also provide liquefaction mitigation. For more information regarding rammed aggregate piers, contact Steve Weyda of Geopier (262-628-1663).

5. 2. 3. Deep Foundations

Either piling or drilled shafts bearing on the less weathered bedrock at the terminal boring depths may be used for support, but further exploration of the bedrock profile should be considered for deep foundations at this site because the bedrock depth and degree of weathering varied between borings. The bedrock depth and degree of weathering have also been found to vary significantly over short distances in other areas around Stevens Point.

If piling is used, the piling should be driven to refusal into the bedrock. Maximum capacities for most driven piles are expected to be achieved in the bedrock at this site

near the depth of auger refusal in the borings, which varied from 32 feet in Boring 3 to 19.5 feet in Boring 4.

If caissons are used, caissons should be drilled to refusal in the bedrock. Because of the sandy soils and shallow water table at this site, drilled shafts should be cased and no belling should be considered. If casing is omitted or belling is used, the shaft will likely collapse, and adjacent locations will be undermined as soil migrates horizontally into the hole. A slurry replacement method may also be necessary for drilled shafts if soil and water seeps into the casing after the casing has been socketed into the rock. Recommended end bearing is 8 tons per square foot for design of caissons drilled to refusal in the bedrock. The base of each caisson should be thoroughly cleaned and, where possible, verified to be free of loose rubble/soils. Any verification of the caisson must follow OSHA regulations. The caisson drill must be capable of removing rock pieces of a diameter up to one-third of the diameter of the caisson. For a higher bearing pressure recommendation at a caisson base, a small diameter caisson load test may be performed. Loading to at least twice the design pressure value is recommended during the test. Caissons bearing on similar rock have been used at other buildings at the UWSP campus, such as the UWSP Fine Arts building. Construction records for these buildings may help in selecting a bearing value.

5. 2. 4. Floor Slab Support

The soils at this site are expected to provide variable support for a floor slab. After excavation for the floor, proof-roll the soils in the floor area. Proof-rolling will increase the density of the soils, providing more uniform support, and help to identify weak soils that are not suitable for support of the floor. An acceptable proof-roller for granular soil would be a smooth-drum, vibratory roller. Consider retaining NTS to observe the proof-rolling. Plan to undercut where weak soils are found. A subgrade modulus of 230 pounds per square inch per inch may be used for sand subgrades at this site which do not yield to the proof-roller.

Proof-rolling of soils near the water table at this site may cause the soils to become quick (liquefaction). The water table should be dropped at least 18 inches below the exposed grade prior to proof-rolling.

As an alternative to proof-rolling and undercutting, the floor slab may be structurally supported.

5. 2. 5. Corrosion Potential

Any construction materials that will be placed in contact with organic soils should be protected against corrosion.

5. 3. Basement Drainage System

Construction of a basement is not recommended at this site because, with water at depths of 3.0 to 6.0 feet (elevation 1085.12 to 1085.67), the potential for flooding of a below-ground level is high.

Should a below-ground level be constructed, it should be constructed at the highest elevation possible and an extensive underdrain system should be installed. Pumps for the underdrain system should be high-capacity pumps capable of running continuously, and a backup power source should be considered to run the pumps should the main power be lost. Drain tile for the system should be spaced at a frequency of no less than one drain per 15 feet, and all drain tiles should be part of one system. Several inches of sand conforming to ASTM C33 should be used as a filter around the drain tile and as the layer just beneath the floor. Native sand in the Stevens Point area is generally too fine and does not meet the ASTM C33 criteria. However, some native sand encountered at this site contained coarser material and may meet this requirement. A gradation test of sand proposed as filter sand would need to be performed to verify the sand's conformance. Clear stone or pea gravel does not work as a filter, and, although some codes require gravel for drains, neither stone nor gravel should be used for drains at this site. Also consider sealing the walls and floor to prevent moisture entering the basement through the walls and floor.

Regardless of actions taken, water problems should be expected at some point in the future in building levels at or below the water table.

5. 4. Compaction and Fill Requirements

The existing sand (both fill and native) encountered, if unsaturated and free of deleterious materials, is acceptable for use as structural fill. The peat encountered should not be used as structural fill. If imported fill is required, we recommend the fill be clean, unsaturated, granular soil. At the time of construction, NTS or another qualified soils engineering firm should verify that the proposed fill soils are acceptable. This firm should verify that the moisture content is appropriate for proper compaction and that the fill contains no deleterious materials. Frozen soil should not be used as structural fill.

Any required fill should be placed in lifts not exceeding 1 foot (uncompacted).

It is recommended that the water table be dropped prior to compaction because compaction of loose sand near or below the water table may cause the sand to become quick (liquefaction). If compaction of the sand near the water table is necessary, use light compaction equipment. To achieve required compaction levels with light equipment, lifts thinner than 1 foot may be necessary.

Compact all fill placed to at least 95% of the maximum dry density (modified Proctor method - ASTM D-1557). Site or soil conditions at the time of construction may warrant a change in the recommended compaction levels. However, no changes should be made without review by NTS or another qualified soils engineering firm.

5. 5. Excavation

All excavations should comply with OSHA regulations. Excavation sidewalls in the on-site sands should have a slope no greater than 2.0 to 1.0. To minimize sidewall cave-in during construction, particularly during vibratory compaction, reduce the sidewall slope to approximately 30 degrees from the horizontal. Where steeper sidewall slopes are required or more convenient, temporary shoring will probably be necessary. Temporary shoring is generally left up to the contractor to design and construct.

Most common excavators (backhoes) should be adequate to make the necessary excavations.

Undercutting for non-structurally-supported floors and shallow foundations should be expected at this site. When undercutting, use the sixty-degree approximation to determine the resulting pressure at the base of the undercut. The recommended width of undercut is twice the undercut depth plus the width of the footing or floor. If the footing/floor location is accurately marked and centered in the base of the undercut, then the minimum width of the undercut is the depth of undercut below the footing plus the width of structure, measured at the base of the undercut. A good practice is to add at least one foot to this width. Replace all undercut soils with properly compacted fill (see section 5. 4. 'Compaction and Fill Requirements'). Compact any disturbed soil at the base of excavations/undercuts with a high energy compactor such as a hoe-pack or vibratory roller.

Excavations should be performed, where possible, using a flat plate attached to the bucket teeth of the backhoe to minimize disturbance at the base of the excavations. Where a toothed bucket is used, the last six inches or so should be excavated by turning the bucket so that the teeth are parallel to the proposed grade. Compact the base of all excavations.

Expect to encounter groundwater in excavations near and below elevation 1085.7. Dewatering will probably be required where water is encountered.

5. 5. 1. Excavation Dewatering

Dewatering will probably be necessary for excavations that extend below the water table. Where excavations are anticipated to extend below the water table, dewatering prior to excavation is recommended. When dewatering, avoid pumping water from

within the excavation. Pumping from within the excavation may loosen the soil at the base of the excavation as surrounding water flows into the excavation. This loosening may not be obvious during construction and can cause significant settlement of foundations. To avoid loosening soil at the base of the excavation, dewater with deep wells or well points from outside the excavation.

It is recommended that the water table be dropped at least 18 inches below the base of the excavation. Should the base of any excavation be at or slightly below the water table, roughly twelve inches of well graded, breaker-run rock (less than 10 percent P200 material) should be placed at the base of the excavation to provide a stable working platform and help prevent loosening of soils at the base of the excavation. The breaker-run should be placed and compacted immediately following excavation, and excavation before breaker-run placement should be limited to small areas.

5. 5. 2. Existing Structures

Care must be taken when excavating next to existing structures. Do not excavate soil under a line drawn out (away from existing structure) and down from the top of a footing at a 45-degree angle, unless proper precautions are taken. If excavations will extend below the elevation of the existing footings, the existing soil under the structure may have to be shored. This may be done using sheet piling, properly braced or tied back. Allow for imposed lateral loads from nearby footings in designing the system. Be aware that vibrations during driving of sheet piles may cause loose soils below the existing structure to settle. Monitor the existing structure for possible movement during the construction process. Construction records for the existing structure should be reviewed, if available. These may show engineered compacted granular fill below and outside existing footings. If this is the case, risk and severity of problems may be reduced if excavation below the elevation of the existing footing is required. Excavating small sections at a time may reduce the potential for seriously undermining the existing footing, and may allow construction to continue without the need to brace or use sheet piles.

Caution should be used when compacting adjacent to existing footings. Vibratory compacting equipment may cause some settling in adjacent soils below existing foundations. It may be better to use light compaction equipment near existing footings.

5. 6. Lateral Earth Pressures

Unsaturated, firm, clean sand (unsaturated on-site sands and imported fill) will exert approximately 35 pounds per cubic foot (pcf) equivalent fluid pressure in the active state, approximately 330 pcf equivalent fluid pressure in the passive state, and approximately 55 pcf equivalent fluid pressure at rest.

Be aware that a saturated soil may exert up to three times as much pressure as a non-saturated soil, and successive passes of a compactor near earth-retaining structures may build up pressures significantly higher than those presented above.

5. 7. Coefficient of Static Friction for Sliding Analysis

For cast-in-place concrete against the sands at this site, a coefficient of static friction of not more than 0.577 should be used in conjunction with a safety factor against sliding of 1.5.

For pre-cast concrete against the sands at this site, a coefficient of static friction of not more than 0.462 should be used in conjunction with a safety factor against sliding of 1.5.

5. 8. Site Classification for Seismic Design

Most sand at this site is loose and below the water table, making the sand potentially liquefiable. According to Table 1615.1.1 of the 2002 Wisconsin Enrolled Commercial Building Code, the site classification for this site would be Site Class 'F' because of these potentially liquefiable soils.

5. 9. Pavement Design Considerations

The following section may be used in the design of any pavement at this site.

It is anticipated that the subgrade soils for the parking lot will consist of either the on-site sand fill or imported clean, fine sand fill. The on-site sand fill was very loose in some areas. It is recommended that the pavement subgrade be thoroughly compacted prior to base course placement.

The sand is slightly frost susceptible and has a Frost Group Designation of F-2. A CBR test was not performed. However, the CBR factor for this material based on the soil description is estimated to be 15.0. The Wisconsin Asphalt Pavement Association Design Guide suggests a soil support value of 5.0 for soils with a CBR value between 10 and 20, classifying the subgrade as 'good'. A subgrade modulus of not more than 230 pounds per square inch per inch should be used for pavement and slab design on these soils.

If asphaltic pavement is used, design thicknesses will vary with expected traffic loads. The traffic load refers to the expected design daily ESALs (18,000 pound Equivalent Single Axle Loads). The following suggested pavement and crushed aggregate base thicknesses are based on 1 to 5 design daily ESALs (usually residential streets, car parking lots, and similar traffic loads).

5. 9. 1. Flexible Pavement, 1 to 5 Design Daily ESALs
Use a minimum of 8 inches of crushed aggregate base course with a minimum of 3 inches of asphaltic concrete.

If the expected daily ESALs are greater, plan to increase these thicknesses.

Rigid (Portland cement concrete) pavement should be used at locations of trash dumpsters or other heavy-duty traffic locations. A thickness of at least 8 inches is recommended for concrete slabs. The base course thickness below the slab is not critical, but it should be thick enough to provide stability for the slab.

The pavement construction should meet the requirements of the Wisconsin DOT Standard Specifications for Road and Bridge Construction.

A prime requirement for successful pavement is preparation of the subgrade soil. At the time the base course is being place, the subgrade should be firm when proof-rolled. Proof-rolling of the proposed pavement subgrade will help identify areas containing peat or other weak soils that may require undercutting. An acceptable proof-roller for sand would be a large, smooth-drum, vibratory compactor. The subgrade may yield slightly to the proof-roller, but after base course placement the base grade should be unyielding to the fully loaded, tandem-axle, dump trucks. This requirement also applies after the completion of any undercut.

It may be necessary to undercut and replace soft or loose soils with crushed rock or breaker-run rock. Any rock used to stabilize a soft subgrade should not be considered as part of the base course thickness.

Respectfully,

Benjamin K. Nummelin

Clifton E.R. Lawson /mn

Benjamin K. Nummelin, E.I.T.
Nummelin Testing Services, Inc.

Clifton E.R. Lawson, P.E.
Consulting Engineer

bkn/cerl/bn/mn

NTS 141.04

NUMMELIN TESTING SERVICES, INC

GEOTECHNICAL ENGINEERING REPORT INFORMATION SHEET

Subsurface soil conditions are responsible for many of the construction problems encountered at building sites. In order to help you, our client, manage your risks, we offer you the following information and suggestions.

Geotechnical engineering reports are based on observations of specific soil conditions existing at the time of the subsurface soil investigation. As these conditions may change over time, construction decisions should be made with the timeliness of the report in mind. Further testing may be advisable if subsurface soil conditions are affected by natural events (flooding, spring thaws, etc.) and construction (drilling, blasting, surcharges, etc.) on-site or adjacent to it. Talking to your geotechnical professional before construction begins will help keep one informed if further tests are recommended.

The recommendations included in your geotechnical engineering report are based on a limited number of samples/tests. These recommendations assume that subsurface conditions throughout the site will be similar to those observed. As all recommendations are preliminary when based on limited testing, it is important to have your geotechnical professional observe the actual conditions during construction. This allows him/her to note any differences that may not have been revealed by the limited samples/tests and/or that are more abrupt than reported in the preliminary report. It is this geotechnical professional, using his/her knowledge and familiarity of site history, as well as construction observations, who will be able to determine if there is adequate and appropriate support to consider these recommendations final. He/she will also be able to document that the contractor is following these recommendations. Be aware that this geotechnical professional can not assume responsibility and/or liability for his/her recommendations based on observations and determinations by others.

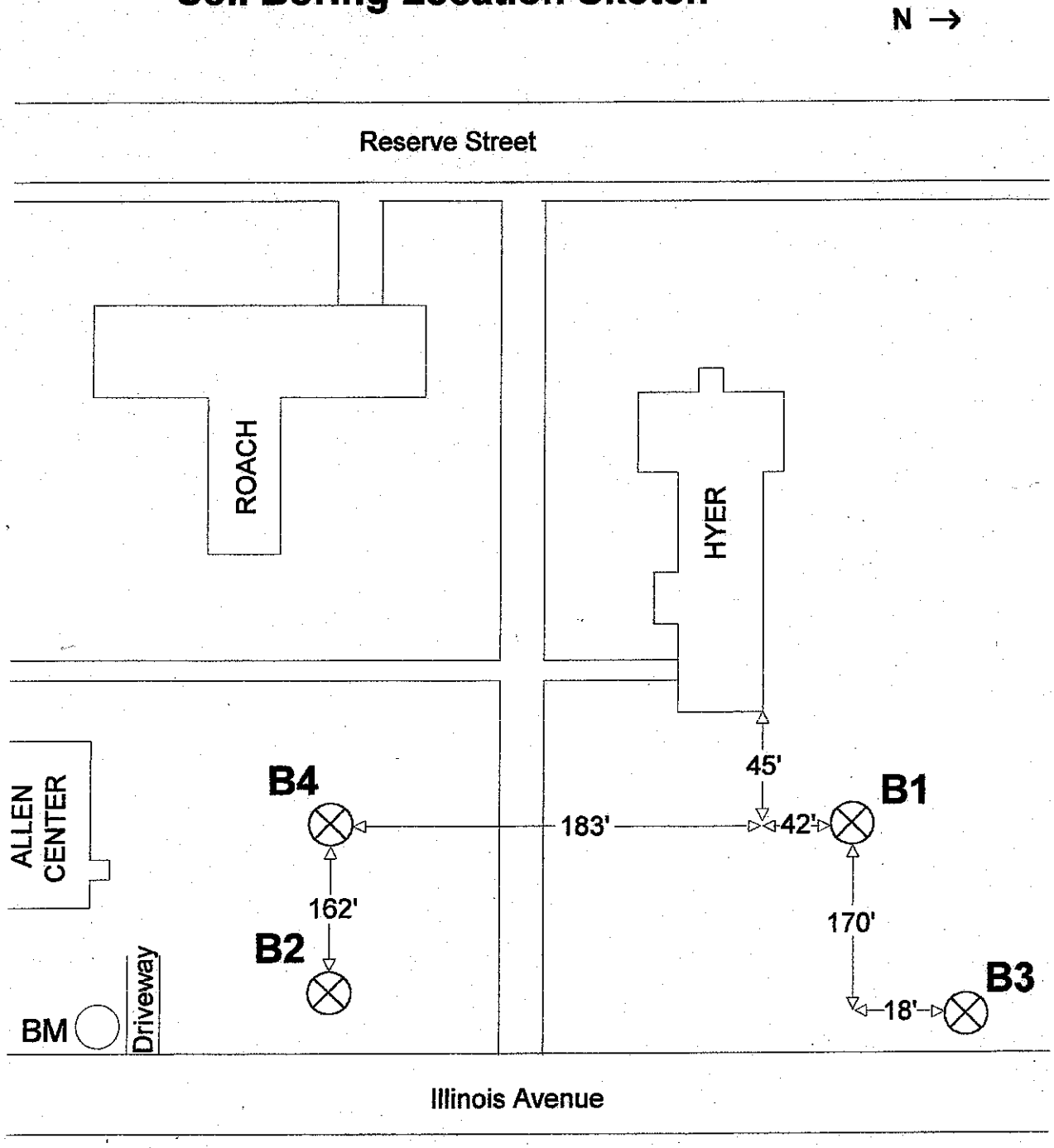
Professional judgement, based on experience and observations, is at the heart of our geotechnical recommendations. Geotechnical reports use information from a limited number of samples/tests to predict conditions regarding your overall site. No one may say with certainty what subsurface conditions really exist without actual observation. The conditions away from sample/test areas may vary from what is predicted. It is important to identify variations as early as possible. This is why we encourage you to take advantage of our knowledge and experience during the construction phase of your project. Working together we can help minimize the impact when unexpected variations occur.

Geotechnical reports are written for a specific client, purpose, project and set of conditions. They are not intended to be a generalized, generic report for a proposed site. They are for the sole use of our client for the express purpose indicated to us. Should the scope of the project be altered, or if subsurface variations become evident during construction, it may be necessary to modify our recommendations. Early communication with your geotechnical professional can help you avoid expensive problems that may occur when changes to a project's purpose, structure, size, usage, site orientation, elevation, etc. are made after a report is written.

Following these guidelines, your geotechnical subsurface report should provide informed and accurate information to assist in the planning and construction of your project.

PROJECT: UWSP Residence
CLIENT: Engberg Anderson
PROJECT # : 141.04
DATE: 07-10-08

Soil Boring Location Sketch



Drawing NOT to Scale
Locations Approximate

BY: BKN
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BORING LOG NOTES

DESCRIPTIVE TERM, GRANULAR SOIL (% BY DRY WEIGHT)

Trace	0% - 5%
Little	5% - 12%
Some	12% - 35%
And	35% - 50%

Q_P = Estimated Unconfined Compressive Strength (by pocket penetrometer) expressed in tons per square foot (t/sf).

Q_U = Estimated Unconfined Compressive Strength (by ASTM 2166) expressed in tons per square foot (t/sf).

NM = Natural Moisture

M = MOISTURE

D = Dry
M = Moist
S = Saturated
F = Frozen
W = Wet

LOI = Loss on Ignition (Organic Content)

N (Standard Blow Count) = blows per foot, as shown. Performed in general accordance with Standard Penetration Test Specifications (ASTM D-1586).

NR = No Recovery
WOH = Weight of Hammer
# = Sample Number

SOIL CLASSIFICATION

F = Fine
M = Medium
C = Coarse
W.L. = Water Level

LL = Liquid Limit, percent
PL = Plastic Limit, percent
PI = Plasticity Index (LL-PL)

SOIL STRENGTH CHARACTERISTICS

CONSISTENCY (Cohesive Soils)

Term	Q_u tons/sq.ft.
Very Soft	0.0 to 0.25
Soft	0.25 to 0.50
Firm	0.50 to 1.0
Stiff	1.0 to 2.0
Very Stiff	2.0 to 4.0
Hard	Over 4.0

RELATIVE DENSITY (Granular Soils)

Term	"N" Value
Very Loose	0 - 4
Loose	4 - 10
Medium-Dense	10 - 30
Dense	30 - 50
Very Dense	Over 50

ORGANIC CONTENT BY COMBUSTION METHOD

Soil Description	Loss On Ignition
Non Organic	Less than 4%
Organic Silt / Clay	4 - 12%
Sedimentary Peat	12 - 50%
Fibrous & Woody Peat	More than 50%

PLASTICITY

Term	Plastic Index
None to Slight	0 - 4
Slight	5 - 7
Medium	8 - 22
High to Very High	Over 22

bornotes.bor

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Residence Hall
Location: Moved 18'S of Proposed Location b/c of Trees - See Plan
Illinois Avenue, Stevens Point, WI 54481

Boring: 1
Auger: 2 1/4" HSA
Page: 1 of 1
Drillers: NH / MA
Date: 7/7/08
Elevation: 1089.47

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty Fine SAND w/ Organics	1	0' - 2'	30	10"	M		Cobble
1	-	(USCS - SM, USDA - Sandy Silt)							
	-	------ 4" ------							
2	-	Black Fine - Medium SAND, Clay Seams, Organics							
	-	Cobbles (Fill) (USCS - SP, USDA - Sand)							
3	-	------ 3.0' ------							
	-		2	3.5' - 5'	9	18"	S		
4	-								
	-								
5	-								
	-								
6	-		3	6' - 7.5'	8	18"	S		
	-								
7	-								
	-								
8	-	Grayish Brown Fine - Medium SAND							
	-	(USCS - SP, USDA - Sand)							
9	-		4	9' - 10.5'	11	18"	S		
	-								
10	-								
	-								
11	-								
	-								
12	-								
	-								
13	-								
	-		5	13.5' - 15'	4	10"	W		1.0
14	-	------ 14.0' ------							
	-	Reddish Brown Sandy CLAY, Little Gravel							
15	-	(Possibly Weathered From Granite)							
	-	(USCS - CL, USDA - Clay Loam)							
16	-	------ E.O.B. 15' ------							
	-	------ Water at Completion 4.3' ------							
17	-	------ Cave-in @ 4.4' ------							
	-	------ Bore Hole filled with Bentonite Chips ------							
18	-								
	-								
19	-								
	-								
20	-								

Nummelin Testing Services, Inc. **NTS #** **141.04**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Residence
Location: Moved 8'W of Proposed Location b/c of Trees - See Plan
Illinois Avenue, Stevens Point, WI 54481

Boring: 2
Auger: 2 1/4" HSA
Page: 1 of 1
Drillers: NH / MA
Date: 7/7/08
Elevation: 1090.92

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty Fine SAND w/ Organics	1	0' - 2'	21	24"	M		
1	-	(USCS - SM, USDA - Sandy Silt)							
	-	------ 8" ------							
2	-	Brown Fine - Medium SAND, Little Gravel (Fill)							
	-	(USCS - SP, USDA - Sand)							
3	-	------ 3.0' ------							
	-		2	3.5' - 5'	10	18"	M		
4	-								
	-								
5	-								
	-								
6	-		3	6' - 7.5'	9	18"	W		
	-								
7	-								
	-								
8	-								
	-								
9	-	Grayish Brown Fine - Medium SAND	4	9' - 10.5'	4	18"	S		
	-	(USCS - SP, USDA - Sand)							
10	-								
	-								
11	-								
	-								
12	-								
	-								
13	-								
	-		5	13.5' - 15'	8	18"	S		
14	-								
	-								
15	-								
	-	------ E.O.B. 15' ------							
16	-	------ Water at Completion 5.8' ------							
	-	------ Cave-in @ 5.9' ------							
17	-	------ Bore Hole filled with Bentonite Chips ------							
	-								
18	-								
	-								
19	-								
	-								
20	-								

Nummelin Testing Services, Inc. **NTS #** **141.04**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Residence Hall
Location: See Plan
Illinois Avenue, Stevens Point, WI 54481

Boring: 3
Auger: 3 1/4" HSA
Page: 1 of 2
Drillers: NH / MA
Date: 7/7/08
Elevation: 1088.67

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Sandy SILT w/ Organics	1	0' - 2'	4	24	M		
1	-	(USCS - ML, USDA - Sandy Silt)							
	-	------ 6" ------							
2	-	Black Sed. PEAT (USCS - Pt, USDA - Muck)							
	-	------ 2.0' ------							
3	-								
	-		2	3.5' - 5'	8	18"	W		
4	-								
	-								
5	-								
	-								
6	-		3	6' - 7.5'	2	18"	S		
	-								
7	-								
	-								
8	-	Grayish Brown Fine - Medium SAND							
	-	(USCS - SP, USDA - Sand)							
9	-		4	9' - 10.5'	8	18	W		
	-								
10	-								
	-								
11	-								
	-								
12	-								
	-								
13	-								
	-								
14	-		5	14' - 15.5'	2	10"	W		
	-								
15	-								
	-								
16	-								
	-	(Harder Drilling @ 17')							
17	-	------ 17.0' ------							
	-								
18	-	Reddish Brown Sandy CLAY, Little Gravel							
	-	(Possibly Weathered From Granite)							
19	-	(USCS - CL, USDA - Clay Loam)	6	19' - 20.5'	8	10	W	2.25	
	-								
20	-								

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Residence Hall
Location: See Plan
Illinois Avenue, Stevens Point, WI 54481

Boring: 3
Auger: 2 1/4" HSA
Page: 2 of 2
Drillers: NH / MA
Date: 7/7/2008
Elevation: 1088.67

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
21		Reddish Brown Sandy CLAY, Little Gravel							
22		*(Possibly Weathered From Granite)*							
23		(USCS - CL, USDA - Clay Loam)							
24			7	24' - 25.5'	18	10	W		
25									
26		(Very Hard Drilling @ 26.5')							
27		------ 26.5' ------							
28		Weathered Granite Bedrock							
29		(USCS - N/A, USDA - Weathered Rock)	8	29' - 30.5'	50/2	18"	M		
30									
31		(Auger Refusal @ 32')							
32		------ E.O.B. 32' ------							
33		------ Water at Completion 3.0' ------							
34		------ Cave-in @ 3.5' ------							
35		------ Bore Hole filled with Bentonite Chips ------							
36									
37									
38									
39									
40									

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Residence Hall
Location: See Plan
Illinois Avenue, Stevens Point, WI 54481

Boring: 4
Auger: 3 1/4" HSA
Page: 1 of 1
Drillers: NH / MA
Date: 7/7/08
Elevation: 1091.47

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty Fine SAND w/ Organics	1	0' - 2'	14	24	M		
1	-	(USCS - SM, USDA - Sandy Silt)							
	-	------ 6" ------							
2	-	Brown Fine to Medium SAND, Some Gravel							
	-	(Fill) (USCS - SP, USDA - Sand)							
3	-	------ 3.0' ------							
	-		2	3.5' - 5'	2	18"	W		
4	-								
	-	Black Sedimentary PEAT							
5	-	(USCS - Pt, USDA - Muck)							
	-								
6	-		3	6' - 7.5'	2	18"	S		
	-								
7	-	------ 7.0' ------							
	-								
8	-								
	-								
9	-		4	9' - 10.5'	8	18	W		
	-	Grayish Brown Fine - Medium SAND							
10	-	(USCS - SP, USDA - Sand)							
	-								
11	-								
	-								
12	-								
	-								
13	-	------ 13.0' ------							
	-								
14	-		5	14' - 15.5'	8	10"	M	1.75	
	-	Grayish Green Silty CLAY							
15	-	(Possibly Weathered From Granite)							
	-	(USCS - CL, USDA - Clay Loam)							
16	-								
	-	(Very Hard Drilling @ 17')							
17	-	------ 17.0' ------							
	-	Weathered Granite Bedrock							
18	-	(USCS - N/A, USDA - Weathered Rock)							
	-	(Auger Refusal @ 19.5')							
19	-	------ E.O.B. 19.5' ------	6	19' - 20.5'	50/1	1	W		
	-	---- Water at Completion 6.0' & Cave-in @ 8' ----							
20	-	------ Bore Hole filled with Bentonite Chips ------							

Nummelin Testing Services, Inc. **NTS #** **141.04**

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03) Page 1

Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between $10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

Route To:

☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
1		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: ☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Feet ☐ N ☐ E ☐ S ☐ W

Latitude: DEG MIN SEC N

Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well:

2. Facility / Owner Information

Facility Name: UWSP Residence Hall

Facility ID	License/Permit No.	City, Village, or Town
140.04		Stevens Point

Street Address of Well: Illinois Avenue

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City	State	ZIP Code

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 7/7/08

If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	4.3'

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal wt.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole

Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	4.3'		

6. Comments

Borehole collapsed @ 4.3 feet when completed.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	07/07/08
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

Signature of Person Doing Work: | Date Signed:

DNR Use Only

Date Received: | Noted By:

Comments:

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

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Route To:

☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
2		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township N	Range ☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W — ☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Latitude: DEG MIN SEC N — Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: _____

2. Facility / Owner Information

Facility Name: UWSP Residence Hall

Facility ID	License/Permit No.	City, Village, or Town
140.04		Stevens Point

Street Address of Well: Illinois Avenue

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City	State	ZIP Code

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 7/7/08

If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	5.8'

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal wt.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	5.9'		

6. Comments

Borehole collapsed @ 5.9 feet when completed.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	07/07/08
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

Signature of Person Doing Work: | Date Signed:

DNR Use Only

Date Received: | Noted By:

Comments:

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03) Page 1

Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between $10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

Route To:

☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
3		Portage

Common Well Name: | Govt Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W ☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: _____

2. Facility / Owner Information

Facility Name: UWSP Residence Hall

Facility ID	License/Permit No.	City, Village, or Town
140.04		Stevens Point

Street Address of Well: Illinois Avenue

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City	State	ZIP Code

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 7/7/08

If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	3.0'

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal wt.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole

From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	3.5'		

6. Comments

Borehole collapsed @ 3.5 feet when completed.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	07/07/08
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

Signature of Person Doing Work: | Date Signed:

DNR Use Only

Date Received	Noted By
Comments	

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

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Route To:

☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
4		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W

☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N

Longitude: DEG MIN SEC W

Reason For Abandonment:

WI Unique Well No. of Replacement Well:

2. Facility / Owner Information

Facility Name: UWSP Residence Hall

Facility ID	License/Permit No.	City, Village, or Town
140.04		Stevens Point

Street Address of Well: Illinois Avenue

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City	State	ZIP Code

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 7/7/08

If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	6.0'

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal wt.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	8.0'		

6. Comments

Borehole collapsed @ 8.0 feet when completed.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	07/07/08
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

Signature of Person Doing Work: | Date Signed:

DNR Use Only

Date Received	Noted By
Comments	

NTS NUMMELIN
TESTING SERVICES, INC.

CENTRAL WISCONSIN AREA:
3217 Whiting Avenue
P.O. Box 127
Stevens Point, WI 54481 -0127
(715) 341-7974 • Fax (715) 341-8654

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SUBSURFACE SOIL INVESTIGATION

UWSP HYER HALL
201 RESERVE STREET
STEVENS POINT
WISCONSIN

NTS 142.80

C09008

PREPARED FOR:

ENGBERG ANDERSON
1 NORTH PINCKNEY STREET
P.O. BOX 100
MADISON, WI 53703-0100

ATTENTION: MR. JAMES BROWN

FIELD INVESTIGATION BY:

NUMMELIN TESTING SERVICES, INC.
3217 WHITING AVENUE
STEVENS POINT, WI

MARCH 2, 2009

SUBSURFACE SOIL INVESTIGATION

UWSP HYER HALL
201 RESERVE STREET
STEVENS POINT
WISCONSIN

1. INTRODUCTION

Nummelin Testing Services, Inc. (NTS) performed this investigation to provide design information for the proposed University of Wisconsin – Stevens Point (UWSP) Hyer Residence Hall to be built at 201 Reserve Street in the City of Stevens Point, Portage County, Wisconsin. The results and recommendations reported are based upon information obtained during a field investigation with borings, and the geotechnical analysis of that information. This investigation follows a preliminary investigation that was done by NTS at this site in July of 2008. The report for the preliminary investigation was titled "Subsurface Investigation, Proposed UWSP Residence Hall, Illinois Avenue, Stevens Point, WI – NTS 141.04," and dated July 16, 2008. This report should be considered in its entirety during design and construction; no one section of the report should be used without considering all other sections.

The conclusions and recommendations reported are based on our interpretation of available subsurface and project information. The report may not represent variations that occur between or away from boring locations.

Should the scope of this project be altered, or if subsurface variations become evident during construction, it may be necessary to modify our recommendations. See the attached Geotechnical Engineering Report Information sheet for general information on NTS's geotechnical reports.

2. PROJECT DESCRIPTION

The proposed project is the construction of a multi-level, dormitory-type, load-bearing masonry building at 201 Reserve Street, just north of the Allen Center. The east building wing is anticipated to be 6 stories with a lower level floor elevation of 1088. The west building wing is anticipated to be 5 stories with lower level floor elevation of 1093.

The maximum interior wall bearing load is estimated at 30 kips, and the maximum exterior wall bearing load is estimated at 15 kips per linear foot. Shallow foundations are anticipated, although caissons are also under consideration.

It is expected that up to three feet of fill will need to be added in the area of the west wing to bring the site to final grade, and some site grading is anticipated in the area of the east wing.

At the time of the investigation, the surface of the site was covered by four tennis courts, three volleyball courts, and several clusters of trees.

3. FIELD INVESTIGATION

Fifteen standard penetration soil borings (Borings SB1 to SB15) were performed from February 16 to February 18, 2009, at locations shown on the attached sketch. Engberg Anderson determined the proposed boring locations and depths.

NTS located the borings in the field. Borings SB1 and SB14 encountered auger refusal at the proposed locations on obstructions at depths of 3 and 1 feet, respectively, and were moved up to 9 feet from the proposed locations and drilled again. Borings SB4 and SB11 had to be moved up to 23 feet from the proposed locations because trees and utilities prevented access to those boring locations. All other borings were drilled at the proposed locations.

Boring SB1 was proposed to be drilled to a depth of 35 feet, but auger refusal occurred in Boring SB1 at 12 feet on February 16, 2009. As requested, Boring SB1 was advanced by diamond-bit coring using an NQ-type barrel to a depth of 17 feet on February 25, 2009. Borings SB2 and SB3 were proposed to be drilled to a depth of 35 feet but were terminated when auger refusal occurred at depths of 37 and 43 feet, respectively, to provide a better profile of the rock should caissons be used for support. Borings SB4 through SB9, SB12, and SB14 were terminated at the proposed depth of 20 feet. Borings SB10, SB11, SB13, and SB15 were proposed to be drilled to a depth 20 feet, but were terminated at a depth of 25 feet to provide a better profile of the rock.

Representative soil samples were obtained while boring using the Standard Penetration Test (SPT) method according to ASTM Test Procedure D1586, at the depths indicated on the boring logs. Drilling between samples was by the hollow-stem-auger technique. The soils were visually/manually classified in general accordance with ASTM Test Procedure

D2488 by a technician at the time the borings were performed. The soil samples have been examined in the laboratory by this writer and classified according to the USCS and, as requested, the USDA soil classification system. Both the USCS and USDA classifications appear on the logs. Several samples were tested in the lab according to ASTM Test Procedure D4972 to determine pH. Other soil parameters and classifications reported herein have been estimated based on the standard penetration test results, soil descriptions, and experience with soils in this area. No other lab testing has been performed.

After completion of the borings, the bore holes were left open to obtain a 24-hour water level, and a temporary well was installed in Boring SB9. After the water level reading, all bore holes, except for the Boring SB9 bore hole, were backfilled with bentonite chips to comply with WDNR requirements, and the last few inches were patched with auger cuttings. The Boring SB9 bore hole will be backfilled with bentonite chips after the temporary well is removed.

At the time of the investigation, NTS determined ground elevations at the boring locations. The top of the large spout of the fire hydrant at the southeast corner of the site was used as a benchmark. According to the City of Stevens Point Engineering Department, the elevation of this benchmark is 1091.84.

Copies of the soil boring logs and a location sketch are appended to this report.

4. SUBSURFACE CONDITIONS

4. 1. Area Geology

The subsoils in this area are mapped as outwash deposits. Outwash deposits generally consist of stratified sand and/or stratified sand and gravel. The underlying rock is mapped as granitic intrusive rock that is present at depths of less than 50 feet below the average surface terrain. A soil survey of Portage County maps the near-surface soils in this area as Roscommon muck that has been cut and filled at various locations. According to old soil maps, it appears that there used to be a creek near this area.

Note that mapped soil and bedrock conditions are provided for additional information only. Use of a design based on mapped or assumed conditions is not recommended.

4. 2. Soils at the Boring Locations

The general soil profile encountered by Borings SB1 to SB15 at this site was 6 to 10 inches of silty sand with organics (topsoil) over sand fill and/or sedimentary peat to

depths of 3 to 5.5 feet (elevation 1084.19 to 1088.84) over very loose to medium-dense, native sand to depths of 7.5 to 23 feet. Below the sand, stiff to hard clay and medium-dense clayey sand (probably weathered from the underlying granite) were encountered to depths of 16 to 34.5 feet. Weathered granite bedrock ('W Bedrock' in Table 1), was encountered in Borings SB1 through SB4, SB6 through SB9, and SB13 at depths of 7.5 to 34.5 feet (elevation 1055.04 to 1083.79). The degree of weathering of the bedrock varied from place to place and with depth, but generally decreased with depth. Auger refusal occurred in Borings SB1 through SB3 at depths of 12 to 43 feet (elevation 1046.54 to 1079.29). A summary of the soil profile encountered in each boring is presented in Table 1.

Table 1. Soil Profiles in the Borings.

Boring	Surf Elev	Fill	Peat	Native Sand	Clay/Sand	W Bedrock
SB1	1091.29	0' – 3'	–	3' – 7.5'	–	7.5' – 12'
SB2	1089.74	0' – 5.5'	2.5' – 3'	3' – 18'	18' – 26'	26' – 37'
SB3	1089.54	0' – 3'	6" – 3'	3' – 19'	19' – 34.5'	34.5' – 43'
SB4	1091.84	0' – 2.5'	2.5' – 3'	3' – 8'	8' – 16'	16' – 20'+
SB5	1091.24	0' – 2.5'	2.5' – 3'	3' – 7.5'	7.5' – 20'+	–
SB6	1090.64	0' – 2.5'	2.5' – 3'	3' – 8'	8' – 13'	13' – 20'+
SB7	1090.84	0' – 2.5'	2.5' – 3'	3' – 12'	12' – 16.5'	16.5' – 20'+
SB8	1090.44	0' – 2.5'	2.5' – 3'	3' – 13'	13' – 18'	18' – 20'+
SB9	1089.64	0' – 2.5'	2.5' – 3'	3' – 13'	13' – 18'	18' – 20'+
SB10	1091.54	0' – 4.5'	4.5' – 4.7'	4.7' – 13'	13' – 25'+	–
SB11	1090.44	0' – 5'	–	5' – 16'	16' – 25'+	–
SB12	1089.39	0' – 0.7'	0.7' – 3'	3' – 17'	17' – 20'+	–
SB13	1090.24	0' – 3'	–	3' – 20.3'	–	20.3' – 25'+
SB14	1089.34	0' – 0.9'	0.9' – 3'	3' – 18'	18' – 20'+	–
SB15	1088.19	0' – 4'	2.5' – 2.7'	4' – 23'	23' – 25'+	–

Note that Borings SB1 and SB14 encountered obstructions in the fill at depths of 3 and 1 feet, respectively, and had to be redrilled at a nearby location. These obstructions appeared to be small slabs of concrete in the fill at these boring locations.

See individual boring logs for more detailed soil descriptions.

4. 3. Rock Coring Results

As requested, after auger refusal occurred in Boring 1 at 12 feet, Boring 1 was advanced by diamond bit coring to a depth of 17 feet. Results of the coring are shown below.

Recovery Ratio = 0.650 RQD = 0.450

The core was of granitic rock which appeared to have softer layers in between hard layers, and fractures ran vertically as well as horizontally through the core sample.

4. 4. Lab Test Results

As requested, samples from 3 borings were tested in the lab to determine pH according to Method A of ASTM Test Procedure D4972 using distilled water. Test results are presented below.

Boring / Sample	Depth	pH
SB1 / S2	3.5'	7.0
SB2 / S3	6.0'	7.2
SB14 / S2	3.5'	6.3

4. 5. Water Level Measurements

Water was encountered in all bore holes during drilling at depths of 4 to 7 feet (elevation 1083.39 to 1085.64). Where the bore hole had not collapsed above the water table prior to the 24 hour reading, water levels in the bore holes were within a few inches of the level at the time of drilling.

At the location of Boring SB9, where the temporary well was installed, the water level after 24 hours and after 72 hours had not changed.

These water level measurements should be considered as representative of site conditions at the time the measurements were made. Seasonal fluctuations in the groundwater table of up to two feet may occur.

5. DISCUSSION AND RECOMMENDATIONS

5. 1. General

Plan to completely remove all existing pavement, any existing foundations, topsoil, and trees from the building footprint prior to further excavation and site grading.

The soils at this site, with no modifications, are expected to provide variable support for shallow foundations. Peat and fill occurred in Borings SB1 through SB15 to depths of 3 to 5.5 (elevation 1084.19 to 1088.84), and in the preliminary borings at depths of 2 to 7 feet. Expect to do some undercutting to remove peat and/or fill from the base of excavations for shallow foundations, particularly those bearing at elevations of 1084 or higher. The amount of undercutting to remove peat and fill will depend on the proposed depth of the foundations. The native sand encountered below the fill and peat was very loose to medium-dense. Compaction of the native sand will probably be necessary at the base of excavations for shallow foundations.

Water was encountered at elevation 1083.84 to 1085.64 in Borings SB1 to SB15, and at elevation 1085.12 to 1085.67 in the preliminary borings at this site. However, it is expected that the water table may rise by up to 2 feet during periods of high groundwater, up to the proposed first floor elevation of 1088 of the east wing. To avoid water problems in the east wing, consider installing a drainage system below the east wing floor.

Expect dewatering to be necessary for excavations near or below elevation 1086 at this site.

See below for further recommendations.

5. 2. Foundations and Floor Slab Support

Standard spread footings may be used at this site, but be aware that undercutting and/or compacting of existing soils will probably be necessary for shallow foundations. Deep foundations, such as caissons or piling bearing on the granite bedrock may also be used. Note that the depth to harder granite increased from roughly 12 feet at the west end of the building (Boring 1) to 43 feet at the east end of the building (Boring 3). Intermediate foundations (rammed aggregate piers) may also be considered.

A review of construction plans and records, if available, for nearby buildings may be beneficial for determining the most economical foundation type.

It is recommended that all load-bearing soils be inspected by NTS or another qualified soils engineering firm to verify that adequate bearing is present.

If the recommendations in this report are followed, foundation total settlement should not exceed one inch, and foundation differential settlement should not exceed one-half inch.

5. 2. 1. Shallow Foundations

It is recommended that shallow foundations bear on the native sand or on fill meeting the requirements of section 5. 4. that bears on the native sand. Note that the native sand was very loose in some borings, and undercutting/compaction of the native sand should be expected below shallow foundations. A presumptive allowable bearing capacity of 3,000 pounds per square foot (psf) may be used in the design of footings if the soil layer just below the footings is comprised of native sand and/or fill that meets these two requirements: the thickness of the layer must be at least one footing's width or 18 inches, whichever thickness is greater, and all soil in the layer must be compacted to at least 95% of the modified Proctor density (ASTM D-1557). Note that undercutting will likely require dewatering.

Shallow foundations should not bear on the peat or existing fill. Peat and/or fill were encountered in Borings SB1 through SB15 to depths of 3 to 5.5 (elevation 1084.19 to 1088.84), and in the preliminary borings at depths of 2 to 7 feet. Expect to undercut where peat and/or existing fill are found at the base of shallow foundation excavations.

5. 2. 2. Intermediate Foundations (Rammed Aggregate Piers)

NTS is not aware of rammed aggregate piers used for other buildings in Stevens Point, but sites with loose sands and high water table, such as this site, may be a good candidate for rammed aggregate piers. Rammed aggregate piers are constructed by ramming thin layers of aggregate into the ground with a high energy tamper. This forms a dense column of aggregate that offers support for bearing pressures of up to 10,000 psf. Rammed aggregate piers can also provide liquefaction mitigation. For more information regarding rammed aggregate piers, contact Steve Weyda of Geopier (262-628-1663).

5. 2. 3. Caissons

Caissons bearing on the harder bedrock encountered at the end of Borings SB1 through SB3 may be used for support. The depth to harder granite increased from roughly 12 feet at the west end of the building (Boring 1) to 43 feet at the east end of the building (Boring 3). Note that the bedrock depth and degree of weathering have

also been found to vary significantly over short distances in other areas around Stevens Point, and may vary between borings.

Caissons should be drilled to refusal in the granite bedrock. Because of the sandy soils and shallow water table at this site, drilled shafts should be cased and no belling should be considered. If casing is omitted or belling is used, the shaft will likely collapse, and adjacent locations will be undermined as soil migrates horizontally into the hole. A slurry replacement method may also be necessary for drilled shafts if soil and water seeps into the casing after the casing has been socketed into the rock. Based on coring results and experience with rock in this area, the recommended end bearing is 8 tons per square foot for design of caissons drilled to refusal in the bedrock. The base of each caisson should be thoroughly cleaned and, where possible, verified to be free of loose rubble/soils. Any verification of the caisson must follow OSHA regulations. The caisson drill must be capable of removing rock pieces of a diameter up to one-third of the diameter of the caisson. For a higher bearing pressure recommendation at a caisson base, a small diameter caisson load test may be performed. Loading to at least twice the design pressure value is recommended during the test. Caissons bearing on similar rock have been used at other buildings at the UWSP campus, such as the UWSP Fine Arts building. Construction records for these buildings may help in selecting a higher bearing value.

5. 2. 4. Piling

Either driven or augered piling may be used for support.

Driving resistances for maximum structural capacity of piles such as 10x42 H piles and 10.75-inch cast-in-place closed-end-pipe piles are expected to occur near the depths of auger refusal in the borings, which varied from roughly 12 feet at the west end of the building (Boring 1) to 43 feet at the east end of the building (Boring 3).

Where vibration may cause movement of existing adjacent buildings, augered piles may be constructed with little or no vibration. Where no load testing is performed, the recommended end bearing for augered piles bearing on the harder granite is 10 tons per square foot. For a higher augered pile end bearing pressure recommendation, a load test may be performed. Loading to at least twice the design pressure value is recommended during the test.

5. 2. 5. Floor Slab Support

The soils at this site are expected to provide variable support for a floor slab. After excavation for the floor, proof-roll the soils in the floor area. Proof-rolling will increase the density of the soils, providing more uniform support, and help to identify

weak soils that are not suitable for support of the floor. An acceptable proof-roller for granular soil would be a smooth-drum, vibratory roller. Consider retaining NTS to observe the proof-rolling. Plan to undercut where weak soils are found. A subgrade modulus of 230 pounds per square inch per inch may be used for sand subgrades at this site which do not yield to the proof-roller.

Proof-rolling of soils near the water table at this site may cause the soils to become quick (liquefaction). The water table should be dropped at least 18 inches below the exposed grade prior to proof-rolling.

As an alternative to proof-rolling and undercutting, the floor slab may be structurally supported.

5. 2. 6. Corrosion Potential

Results of pH lab tests on the sands at depths of 3 to 6 feet at this site ranged from 6.3 to 7.2, indicating the corrosion potential of steel in the sands to be low to moderate. However, peat and other organic soils were encountered above these depths. The corrosion potential of steel in the organic soils is likely to be high. Any construction materials that will be placed in contact with the organic soils should be protected against corrosion.

5. 3. Drainage System

During periods of high groundwater, the water table may rise up to the proposed first floor elevation of the east wing. To avoid water problems in the east wing, consider installing a drainage system and a vapor barrier below the east wing floor.

Drain tile for the system should be spaced at a frequency of no less than one drain per 15 feet, and all drain tiles should be part of one system. Several inches of sand conforming to ASTM C33 should be used as a filter around the drain tile and as the layer just beneath the floor. Clear stone or pea gravel does not work as a filter, and, although some codes require gravel for drains, neither stone nor gravel should be used for drains at this site.

Do not connect the outlet of a gravity-flow drainage system to any other storm drain systems because water may flow backward through the drains and into the building during periods of heavy stormwater flow. Consider directing the flow to an exterior manhole where the water could be pumped out with a portable pump during periods when the water table is high.

5. 4. Compaction and Fill Requirements

The existing sand (both fill and native) encountered, if unsaturated and free of deleterious materials, is acceptable for use as structural fill. The peat encountered should not be used as structural fill. If imported fill is required, we recommend the fill be clean, unsaturated, granular soil. At the time of construction, NTS or another qualified soils engineering firm should verify that the proposed fill soils are acceptable. This firm should verify that the moisture content is appropriate for proper compaction and that the fill contains no deleterious materials. Frozen soil should not be used as structural fill.

Any required fill should be placed in lifts not exceeding 1 foot (uncompacted).

Compaction of loose sand near or below the water table may cause the sand to become quick (liquefaction). If compaction of the sand near the water table is necessary, drop the water table at least 18 inches below the proposed grade and use light compaction equipment. To achieve required compaction levels with light equipment, lifts thinner than 1 foot may be necessary.

Compact all fill placed to at least 95% of the maximum dry density (modified Proctor method - ASTM D-1557). Site or soil conditions at the time of construction may warrant a change in the recommended compaction levels. However, no changes should be made without review by NTS or another qualified soils engineering firm.

5. 5. Excavation

All excavations should comply with OSHA regulations. Excavation sidewalls in the on-site sands should have a slope no steeper than 1.5 to 1.0. To minimize sidewall cave-in during construction, particularly during vibratory compaction, reduce the sidewall slope to approximately 30 degrees from the horizontal. Where steeper sidewall slopes are required or more convenient, temporary shoring will probably be necessary. Temporary shoring is generally left up to the contractor to design and construct.

Most common excavators (backhoes) should be adequate to make the necessary excavations.

Undercutting for non-structurally-supported floors and shallow foundations should be expected at this site. When undercutting, use the sixty-degree approximation to determine the resulting pressure at the base of the undercut. The recommended width of undercut is twice the undercut depth plus the width of the footing or floor. If the footing/floor location is accurately marked and centered in the base of the undercut, then the minimum width of the undercut is the depth of undercut below the footing plus the width of

structure, measured at the base of the undercut. A good practice is to add at least one foot to this width. Replace all undercut soils with properly compacted fill (see section 5. 4. 'Compaction and Fill Requirements'). Compact any disturbed soil at the base of excavations/undercuts with a high energy compactor such as a hoe-pack or vibratory roller.

Excavations should be performed, where possible, using a flat plate attached to the bucket teeth of the backhoe to minimize disturbance at the base of the excavations. Where a toothed bucket is used, the last six inches or so should be excavated by turning the bucket so that the teeth are parallel to the proposed grade. Compact the base of all excavations.

Expect to encounter groundwater in excavations near and below elevation 1086. Dewatering will probably be required where water is encountered.

5. 5. 1. Excavation Dewatering

Dewatering will probably be necessary for excavations that extend below the water table. Where excavations are anticipated to extend below the water table, dewatering prior to excavation is recommended. When dewatering, avoid pumping water from within the excavation. Pumping from within the excavation may loosen the soil at the base of the excavation as surrounding water flows into the excavation. This loosening may not be obvious during construction and can cause significant settlement of foundations. To avoid loosening soil at the base of the excavation, dewater with deep wells or well points from outside the excavation.

It is recommended that the water table be dropped at least 18 inches below the base of the excavation. Should the base of any excavation be at or slightly below the water table, roughly twelve inches of well graded, breaker-run rock (less than 10 percent P200 material) may be placed at the base of the excavation to provide a stable working platform and help prevent loosening of soils at the base of the excavation. The breaker-run should be placed and compacted immediately following excavation, and excavation before breaker-run placement should be limited to small areas.

5. 5. 2. Existing Structures

Care must be taken when excavating next to existing structures. Do not excavate soil under a line drawn out (away from existing structure) and down from the top of a footing at a 45-degree angle, unless proper precautions are taken. If excavations will extend below the elevation of the existing footings, the existing soil under the structure may have to be shored. This may be done using sheet piling, properly braced or tied back. Allow for imposed lateral loads from nearby footings in designing the system. Be aware that vibrations during driving of sheet piles may cause loose soils

below the existing structure to settle. Monitor the existing structure for possible movement during the construction process. Construction records for the existing structure should be reviewed, if available. These may show engineered compacted granular fill below and outside existing footings. If this is the case, risk and severity of problems may be reduced if excavation below the elevation of the existing footing is required. Excavating small sections at a time may reduce the potential for seriously undermining the existing footing, and may allow construction to continue without the need to brace or use sheet piles.

Caution should be used when compacting adjacent to existing footings. Vibratory compacting equipment may cause some settling in adjacent soils below existing foundations. It may be better to use light compaction equipment near existing footings.

5. 6. Lateral Earth Pressures

Unsaturated, firm, clean sand (unsaturated on-site sands and imported fill) will exert approximately 35 pounds per cubic foot (pcf) equivalent fluid pressure in the active state, approximately 330 pcf equivalent fluid pressure in the passive state, and approximately 55 pcf equivalent fluid pressure at rest.

Be aware that a saturated soil may exert up to three times as much pressure as a non-saturated soil, and successive passes of a compactor near earth-retaining structures may build up pressures significantly higher than those presented above.

5. 7. Coefficient of Static Friction for Sliding Analysis

For cast-in-place concrete against the sands at this site, a coefficient of static friction of not more than 0.577 should be used in conjunction with a safety factor against sliding of 1.5.

For pre-cast concrete against the sands at this site, a coefficient of static friction of not more than 0.462 should be used in conjunction with a safety factor against sliding of 1.5.

5. 8. Site Classification for Seismic Design

Most sand at this site is loose and below the water table, making the sand potentially liquefiable. According to Table 1615.1.1 of the 2002 Wisconsin Enrolled Commercial Building Code, the site classification for this site would be Site Class 'F' because of these potentially liquefiable soils.

5. 9. Pavement Design Considerations

The following section may be used in the design of any pavement at this site.

It is anticipated that the subgrade soils for the parking lot will consist of either the on-site sand fill or imported clean, fine sand fill. The on-site sand fill was very loose in some areas. It is recommended that the pavement subgrade be thoroughly compacted prior to base course placement.

The sand is slightly frost susceptible and has a Frost Group Designation of F-2. A CBR test was not performed. However, the CBR factor for this material based on the soil description is estimated to be 15.0. The Wisconsin Asphalt Pavement Association Design Guide suggests a soil support value of 5.0 for soils with a CBR value between 10 and 20, classifying the subgrade as 'good'. A subgrade modulus of not more than 230 pounds per square inch per inch should be used for pavement and slab design on these soils.

If asphaltic pavement is used, design thicknesses will vary with expected traffic loads. The traffic load refers to the expected design daily ESALs (18,000 pound Equivalent Single Axle Loads). The following suggested pavement and crushed aggregate base thicknesses are based on 1 to 5 design daily ESALs (usually residential streets, car parking lots, and similar traffic loads).

5. 9. 1. Flexible Pavement, 1 to 5 Design Daily ESALs
Use a minimum of 8 inches of crushed aggregate base course with a minimum of 3 inches of asphaltic concrete.

If the expected daily ESALs are greater, plan to increase these thicknesses.

Rigid (Portland cement concrete - PCC) pavement may be used. Below normal parking areas, a slab thickness of at least 6 inches is recommended. PCC pavement should also be used at locations of trash dumpsters or other heavy-duty traffic locations. A thickness of at least 8 inches is recommended for concrete slabs in heavy-duty traffic areas. The base course thickness below the slab is not critical, but it should be thick enough to provide stability for the slab.

The pavement construction should meet the requirements of the Wisconsin DOT Standard Specifications for Road and Bridge Construction.

A prime requirement for successful pavement is preparation of the subgrade soil. At the time the base course is being placed, the subgrade should be firm when proof-rolled. Proof-rolling of the proposed pavement subgrade will help identify areas containing peat or other weak soils that may require undercutting. An acceptable proof-roller for sand would be a large, smooth-drum, vibratory compactor. The subgrade may yield slightly to

the proof-roller, but after base course placement the base grade should be unyielding to the fully loaded, tandem-axle, dump trucks. This requirement also applies after the completion of any undercut.

It may be necessary to undercut and replace soft or loose soils with crushed rock or breaker-run rock. Any rock used to stabilize a soft subgrade should not be considered as part of the base course thickness.

Respectfully,

Benjamin K. Nummelin, P.E.
Nummelin Testing Services, Inc.

Clifton E.R. Lawson, P.E.
Consulting Engineer

bkn/cerl/bn/mn

NUMMELIN TESTING SERVICES, INC

GEOTECHNICAL ENGINEERING REPORT INFORMATION SHEET

Subsurface soil conditions are responsible for many of the construction problems encountered at building sites. In order to help you, our client, manage your risks, we offer you the following information and suggestions.

Geotechnical engineering reports are based on observations of specific soil conditions existing at the time of the subsurface soil investigation. As these conditions may change over time, construction decisions should be made with the timeliness of the report in mind. Further testing may be advisable if subsurface soil conditions are affected by natural events (flooding, spring thaws, etc.) and construction (drilling, blasting, surcharges, etc.) on-site or adjacent to it. Talking to your geotechnical professional before construction begins will help keep one informed if further tests are recommended.

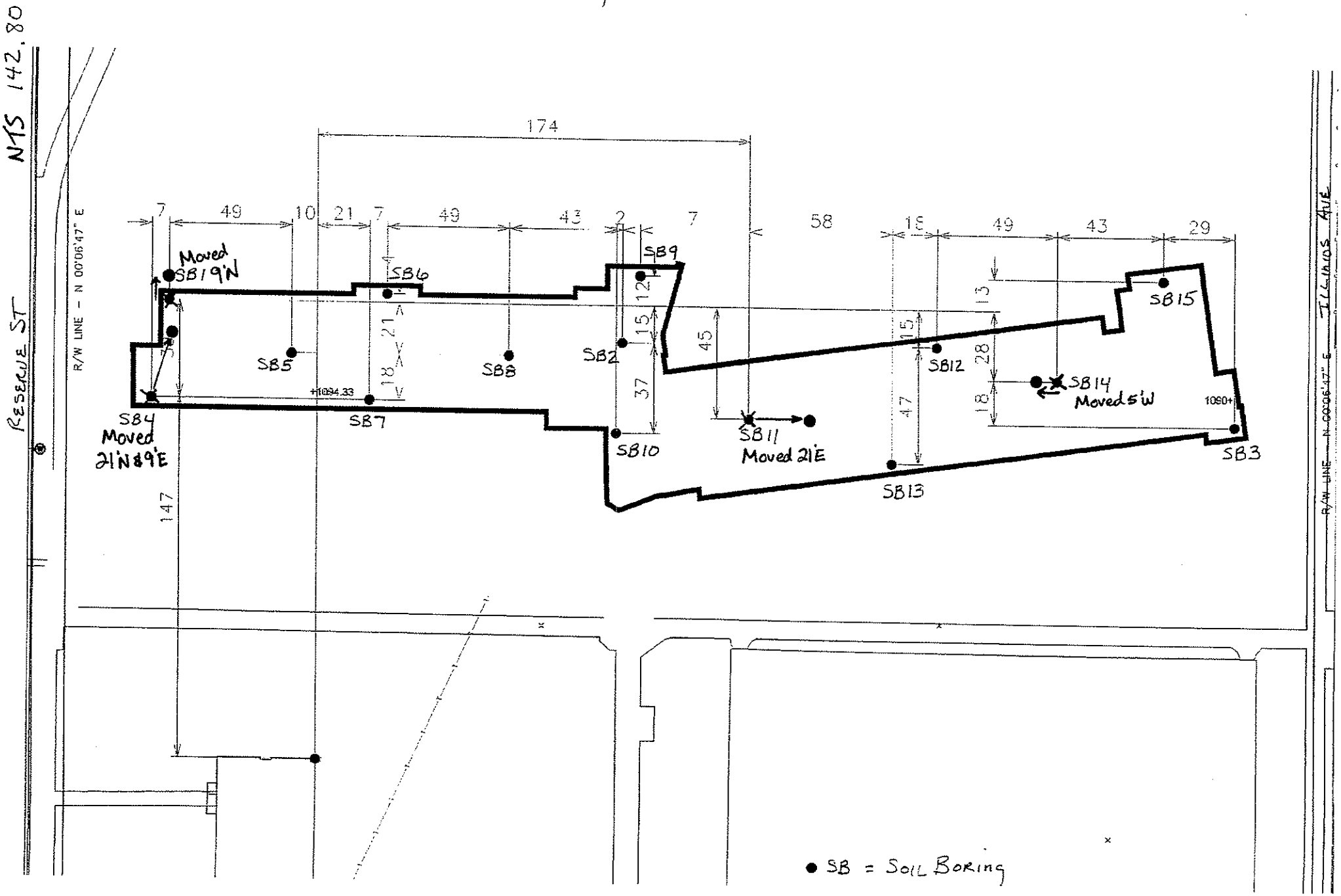
The recommendations included in your geotechnical engineering report are based on a limited number of samples/tests. These recommendations assume that subsurface conditions throughout the site will be similar to those observed. As all recommendations are preliminary when based on limited testing, it is important to have your geotechnical professional observe the actual conditions during construction. This allows him/her to note any differences that may not have been revealed by the limited samples/tests and/or that are more abrupt than reported in the preliminary report. It is this geotechnical professional, using his/her knowledge and familiarity of site history, as well as construction observations, who will be able to determine if there is adequate and appropriate support to consider these recommendations final. He/she will also be able to document that the contractor is following these recommendations. Be aware that this geotechnical professional can not assume responsibility and/or liability for his/her recommendations based on observations and determinations by others.

Professional judgement, based on experience and observations, is at the heart of our geotechnical recommendations. Geotechnical reports use information from a limited number of samples/tests to predict conditions regarding your overall site. No one may say with certainty what subsurface conditions really exist without actual observation. The conditions away from sample/test areas may vary from what is predicted. It is important to identify variations as early as possible. This is why we encourage you to take advantage of our knowledge and experience during the construction phase of your project. Working together we can help minimize the impact when unexpected variations occur.

Geotechnical reports are written for a specific client, purpose, project and set of conditions. They are not intended to be a generalized, generic report for a proposed site. They are for the sole use of our client for the express purpose indicated to us. Should the scope of the project be altered, or if subsurface variations become evident during construction, it may be necessary to modify our recommendations. Early communication with your geotechnical professional can help you avoid expensive problems that may occur when changes to a project's purpose, structure, size, usage, site orientation, elevation, etc. are made after a report is written.

Following these guidelines, your geotechnical subsurface report should provide informed and accurate information to assist in the planning and construction of your project.

Soil Boring Locations



BORING LOG NOTES

DESCRIPTIVE TERM, GRANULAR SOIL (% BY DRY WEIGHT)

Trace	0% - 5%
Little	5% - 12%
Some	12% - 35%
And	35% - 50%

Q_P = Estimated Unconfined Compressive Strength (by pocket penetrometer) expressed in tons per square foot (t/sf).

Q_U = Estimated Unconfined Compressive Strength (by ASTM 2166) expressed in tons per square foot (t/sf).

NM = Natural Moisture

M = MOISTURE

D = Dry
M = Moist
S = Saturated
F = Frozen
W = Wet

LOI = Loss on Ignition (Organic Content)

N (Standard Blow Count) = blows per foot, as shown. Performed in general accordance with Standard Penetration Test Specifications (ASTM D-1586).

NR = No Recovery WOH = Weight of Hammer # = Sample Number

SOIL CLASSIFICATION

F = Fine
M = Medium
C = Coarse
W.L. = Water Level

LL = Liquid Limit, percent
PL = Plastic Limit, percent
PI = Plasticity Index (LL-PL)

SOIL STRENGTH CHARACTERISTICS

CONSISTENCY (Cohesive Soils)

Term	Q_u tons/sq.ft.
Very Soft	0.0 to 0.25
Soft	0.25 to 0.50
Firm	0.50 to 1.0
Stiff	1.0 to 2.0
Very Stiff	2.0 to 4.0
Hard	Over 4.0

RELATIVE DENSITY (Granular Soils)

Term	"N" Value
Very Loose	0 - 4
Loose	4 - 10
Medium-Dense	10 - 30
Dense	30 - 50
Very Dense	Over 50

ORGANIC CONTENT BY COMBUSTION METHOD

Soil Description	Loss On Ignition
Non Organic	Less than 4%
Organic Silt / Clay	4 - 12%
Sedimentary Peat	12 - 50%
Fibrous & Woody Peat	More than 50%

PLASTICITY

Term	Plastic Index
None to Slight	0 - 4
Slight	5 - 7
Medium	8 - 22
High to Very High	Over 22

bornotes.bor

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: Moved 9'N - See Plan
201 Reserve St, Stevens Point, WI

Boring: SB1
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1091.29

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty SAND w/ Organics							
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	18	18	F		
	-	------ 8" ------							
2	-	Dark Brown Fine SAND, Some Silt							
	-	(Poss Fill) (USCS: SM) (USDA: Loamy Sand)							
3	-	(Obstruction@3' - Moved 9'N & Drilled Again)							
	-	------ 3.0' ------	2	3.5 - 5	12	18	M		
4	-								
	-								
5	-	Light Brown Mostly Fine SAND							
	-	(USCS: SP) (USDA: Sand)							
6	-		3	6 - 7.5	7	18	S		
	-								
7	-								
	-	------ 7.5' ------							
8	-								
	-	GRAVEL w/ Cobbles							
9	-	(USCS: GP) (USDA: Gravelly Sand)	4	9 - 10.5	45	3	S		
	-	(Probable Weathered Granite Bedrock)							
10	-	(Hard Drilling @ 10')							
	-								
11	-								
	-	(Auger Refusal @ 12')							
12	-	(Boring Advanced By Diamond Bit Coring)							
	-	(On 02-25-09)							
13	-								
	-								
14	-								
	-								
15	-								
	-								
16	-								
	-	--- Recovery Ratio = 0.650 RQD = 0.450 ---							
17	-	------ E.O.B. 17.0' ------							
	-	------ Borehole Filled with Bentonite Chips ------							
18	-	-- Water @ EOB 5.8', 24hr 5.7' --							
	-								
19	-								
	-								
20	-								
	-								
21	-								
	-								
22	-								
	-								
23	-								
	-								
24	-								
	-								
25	-								

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB2
Auger: HSA
Page: 1 of 2
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1089.74

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
		Dark Brown Silty SAND w/ Organics							
1		(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	7	18	M		
		------ 10" ------							
2									
		(Black Peat Layer 2.5' - 3.0')							
3		Dark Brown Fine SAND, Little Silt							
		(Poss Fill) (USCS: SP-SM) (USDA: Sand)	2	3.5 - 5	2	18	W		
4									
5		------ 5.5' ------							
6			3	6 - 7.5	5	18	S		
7									
8									
9			4	9 - 10.5	7	18	S		
10									
11		Light Brown Mostly Fine SAND							
		(USCS: SP) (USDA: Sand)							
12									
13									
14			5	14 - 15.5	13	18	S		
15									
16									
17									
18		------ 18.0' ------							
19			6	19 - 20.5	13	18	S	1.5	
20									
21		Brownish Red CLAY							
		Little Gravel							
22		(USCS: CH) (USDA: Clay)							
		(Probably Weathered From Granite)							
23									
24			7	24 - 25.5	10	18	S	4.5	
25		(Continued)							

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: 2
Auger: HSA
Page: 2 of 2
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1089.74

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
		(Continued)							
26		------ 26.0' ------							
		(Hard Drilling @ 26')							
27									
28									
29			8	29 - 30.5	34	18	S	4.5	
30		Brownish Red CLAY							
		Little Gravel							
31		(USCS: CH) (USDA: Clay)							
32		(Probable Weathered Granite Bedrock)							
33									
34			9	34 - 35.5	50/3	9	S		
35									
36									
37		(Auger Refusal @ 37')							
		------ E.O.B. 37.0' ------							
38		------ Borehole Filled with Bentonite Chips ------							
		-- Water @ EOB 5.0', 24hr 4.9' --							
39									
40									
41									
42									
43									
44									
45									
46									
47									
48									
49									
50									

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB3
Auger: HSA
Page: 1 of 2
Drillers: NH / MA
Date: 2/17/2009
Elevation: 1089.54

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Brown F - M SAND (Fill)							
1	-	(USCS: SP) (USDA: Sand)	1	1 - 2.5	6	18	M		
	-	------ 6" ------							
2	-	Black Organic SILT							
	-	(Poss Fill) (USCS: Pt) (USDA: Muck)							
3	-	------ 3.0' ------							
	-		2	3.5 - 5	7	18	M		
4	-								
	-	Light Brown F - M SAND							
5	-	(USCS: SP) (USDA: Sand)							
	-								
6	-		3	6 - 7.5	4	18	S		
	-								
7	-	------ 7.0' ------							
	-								
8	-								
	-								
9	-		4	9 - 10.5	5	18	S		
	-								
10	-								
	-								
11	-								
	-								
12	-								
	-	Light Brown Mostly Fine SAND							
13	-	(USCS: SP) (USDA: Sand)							
	-								
14	-		5	14 - 15.5	7	18	S		
	-								
15	-								
	-								
16	-								
	-								
17	-								
	-								
18	-								
	-								
19	-	------ 19.0' ------	6	19 - 20.5	11	18	S		
	-								
20	-								
	-								
21	-	Brown/Orange Clayey SAND							
	-	Some Gravel							
22	-	(USCS: SC) (USDA: Sandy Clay Loam)							
	-	(Probably Weathered From Granite)							
23	-								
	-								
24	-		7	24 - 25.5	10	18	S		
	-								
25	-	(Continued)							

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB3
Auger: HSA
Page: 2 of 2
Drillers: NH / MA
Date: 2/17/2009
Elevation: 1089.54

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
		(Continued)							
26									
27									
28									
29		Brown/Orange Clayey SAND Some Gravel (USCS: SC) (USDA: Sandy Clay Loam)	8	29 - 30.5	20	18	S		
30		(Probably Weathered From Granite)							
31									
32									
33									
34			9	34 - 35.5	60	18	S		
35		------ 34.5' ------							
36									
37									
38		Brown/Orange Clayey SAND Some Gravel (USCS: SC) (USDA: Sandy Clay Loam)							
39		(Probable Weathered Granite Bedrock)	10	39 - 40.5	50/2	8	S		
40									
41									
42									
43		(Auger Refusal @ 43') ------ E.O.B. 43.0' ------							
44		------ Borehole Filled with Bentonite Chips ------ -- Water @ EOB 5.0', 24hr 5.1' --							
45									
46									
47									
48									
49									
50									

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: Moved 21'N and 9'E - See Plan
201 Reserve St, Stevens Point, WI

Boring: SB4
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1091.84

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty SAND w/ Organics							
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	7	18	M		
	-	------ 8" ------							
2	-	(Probable Sand Fill to 2.5')							
	-	(Black Peat Layer 2.5' - 3.0')							
3	-								
	-		2	3.5 - 5	18	3	M		
4	-								
	-	Brown Mostly Fine SAND							
5	-	(USCS: SP) (USDA: Sand)							
	-								
6	-		3	6 - 7.5	13	18	S		
	-								
7	-								
	-								
8	-	------ 8.0' ------							
	-								
9	-		4	9 - 10.5	18	18	S	3.0	
	-								
10	-								
	-								
11	-	Grayish Brown CLAY							
	-	Some Gravel							
12	-	(USCS: CL) (USDA: Clay)							
	-	(Weathered From Granite)							
13	-								
	-								
14	-		5	14 - 15.5	7	18	S	1.5	
	-								
15	-								
	-	(Hard Drilling @ 16')							
16	-	------ 16.0' ------							
	-								
17	-								
	-	Grayish Brown CLAY							
18	-	Some Gravel							
	-	(USCS: CL) (USDA: Clay)	6	18.5 - 20	32	18	S		
19	-	(Probable Weathered Granite Bedrock)							
	-								
20	-	------ E.O.B. 20.0' ------							
	-	------ Borehole Filled with Bentonite Chips ------							
21	-	-- Water @ 6.2', 24hr 6.2' --							
	-								
22	-								
	-								
23	-								
	-								
24	-								
	-								
25	-								

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB5
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1091.24

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty SAND w/ Organics							
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	4	18	M		
	-	------ 10" ------							
2	-	Dark Brown Fine SAND, Little Silt							
	-	(Fill) (USCS: SP-SM) (USDA: Loamy Sand)							
3	-	------ 2.5' ------							
	-	(Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	6	18	M		
4	-								
	-								
5	-	Tan F - M SAND							
	-	(USCS: SP) (USDA: Sand)							
6	-		3	6 - 7.5	8	18	S		
	-								
7	-								
	-	------ 7.5' ------							
8	-								
	-								
9	-		4	9 - 10.5	7	18	S		
	-								
10	-	Grayish Brown F - M Clayey SAND							
	-	(USCS: SC) (USDA: Sandy Clay Loam)							
11	-								
	-								
12	-								
	-								
13	-	------- 13.0' -------							
	-								
14	-		5	14 - 15.5	9	18	S		
	-								
15	-								
	-								
16	-	Brownish Red F - M Silty SAND							
	-	Some Gravel							
17	-	(USCS: SM) (USDA: Loamy Sand)							
	-	(Probably Weathered From Granite)							
18	-								
	-		6	18.5 - 20	20	18	S		
19	-								
	-								
20	-	------ E.O.B. 20.0' ------							
	-	------ Borehole Filled with Bentonite Chips ------							
21	-	-- Water @ EOB 6.0', 24hr Dry (Cave-in 5.9') --							
	-								
22	-								
	-								
23	-								
	-								
24	-								
	-								
25	-								

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB6
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1090.64

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty SAND w/ Organics							
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	6	18	M		
	-	------ 8" ------							
2	-	Dark Brown Fine SAND, Little Silt							
	-	(Fill) (USCS: SP-SM) (USDA: Loamy Sand)							
3	-	------ 2.5' ------							
	-	(Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	8	18	M		
4	-								
	-	Light Brown F - M SAND							
5	-	(USCS: SP) (USDA: Sand)							
	-	------ 5.5' ------							
6	-		3	6 - 7.5	4	18	S		
	-								
7	-	Light Brown Fine SAND							
	-	(USCS: SP) (USDA: Sand)							
8	-	------ 8.0' ------							
	-								
9	-		4	9 - 10.5	24	18	S		
	-								
10	-	Grayish Brown F - M Silty SAND							
	-	(USCS: SM) (USDA: Sandy Loam)							
11	-	(Probably Weathered From Granite)							
	-								
12	-								
	-								
13	-	------ 13.0' ------							
	-								
14	-		5	14 - 15.5	32	18	S	2.0	
	-								
15	-	Brown Sandy CLAY							
	-	(USCS: CL) (USDA: Clay Loam)							
16	-	(Probable Weathered Granite Bedrock)							
	-								
17	-								
	-	------ 17.5' ------							
18	-								
	-	Brown/Red/White SAND and GRAVEL	6	18.5 - 20	43	18	S		
19	-	(USCS: GP) (USDA: Gravelly Sand)							
	-	(Probable Weathered Granite Bedrock)							
20	-	------ E.O.B. 20.0' ------							
	-	------ Borehole Filled with Bentonite Chips ------							
21	-	-- Water @ EOB 6.0', 24hr Dry (Cave-in 5.4') --							
	-								
22	-								
	-								
23	-								
	-								
24	-								
	-								
25	-								

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB7
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1090.84

Depth (ft)	Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
1	Dark Brown Silty SAND w/ Organics (USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	4	18	M		
	------ 8" ------							
2	Dark Brown Fine SAND, Little Silt (Fill) (USCS: SP-SM) (USDA: Loamy Sand)							
3	------ 2.5' ------							
	(Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	6	18	M		
4								
	Light Brown F - M SAND							
5	(USCS: SP) (USDA: Sand)							
6	------ 6.0' ------	3	6 - 7.5	7	18	S		
7								
8								
9	Light Brown Fine SAND (USCS: SP) (USDA: Sand)	4	9 - 10.5	5	18	S		
10								
11								
12	------ 12.0' ------							
13								
14	Brown CLAY (USCS: CH) (USDA: Clay)	5	14 - 15.5	4	18	S	1.0	
15	(Probably Weathered From Granite)							
16								
	------ 16.5' ------							
17								
18	Brown/Red/Orange SAND and GRAVEL (USCS: SP) (USDA: Gravelly Sand)	6	18.5 - 20	42	18	S		
19	(Probable Weathered Granite Bedrock)							
20	------ E.O.B. 20.0' ------							
	------ Borehole Filled with Bentonite Chips ------							
21	-- Water @ EOB 6.0', 24hr 6.1' --							
22								
23								
24								
25								

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB8
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1090.44

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
		Dark Brown Silty SAND w/ Organics							
1		(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	9	18	M		
		------ 10" ------							
2		Dark Brown Fine SAND, Little Silt							
		(Fill) (USCS: SP-SM) (USDA: Loamy Sand)							
3		------ 2.5' ------							
		(Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	11	18	M		
4									
		Light Brown F - M SAND							
5		(USCS: SP) (USDA: Sand)							
6		------ 6.0' ------	3	6 - 7.5	9	18	S		
7									
8									
9			4	9 - 10.5	3	18	S		
		Light Brown Fine SAND							
10		(USCS: SP) (USDA: Sand)							
11									
12									
13		------ 13.0' ------							
14			5	14 - 15.5	8	18	S	3.0	
15		Brownish Red CLAY							
		(USCS: CH) (USDA: Clay)							
16		(Probably Weathered From Granite)							
17									
18		------ 18.0' ------							
		Brown/Red/Orange SAND and GRAVEL	6	18.5 - 20	31	18	S		
19		(USCS: SP) (USDA: Gravelly Sand)							
		(Probable Weathered Granite Bedrock)							
20		------ E.O.B. 20.0' ------							
		------ Borehole Filled with Bentonite Chips ------							
21		-- Water @ EOB 6.0', 24hr Dry (Cave-in 5.4') --							
22									
23									
24									
25									

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan (Temporary Well Installed Here)
201 Reserve St, Stevens Point, WI

Boring: SB9
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1089.64

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty SAND w/ Organics							
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	6	18	M		
	-	------ 10" ------							
2	-	Dark Brown Fine SAND, Little Silt							
	-	(Fill) (USCS: SP-SM) (USDA: Loamy Sand)							
3	-	------ 2.5' ------							
	-	(Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	5	18	M		
4	-								
	-								
5	-								
	-								
6	-		3	6 - 7.5	3	18	S		
	-								
7	-								
	-	Light Brown Fine SAND							
8	-	(USCS: SP) (USDA: Sand)							
	-								
9	-		4	9 - 10.5	7	18	S		
	-								
10	-								
	-								
11	-								
	-								
12	-								
	-								
13	-	------ 13.0' ------							
	-								
14	-		5	14 - 15.5	9	18	S	1.5	
	-								
15	-	Brown CLAY							
	-	Little to Some Sand							
16	-	(USCS: CH) (USDA: Clay)							
	-	(Probably Weathered From Granite)							
17	-								
	-								
18	-	------ 18.0' ------							
	-	Brown CLAY, Little to Some Sand	6	18.5 - 20	38	18	S		
19	-	(USCS: CH) (USDA: Clay)							
	-	(Probable Weathered Granite Bedrock)							
20	-	------ E.O.B. 20.0' ------							
	-	------ Borehole Filled with Bentonite Chips ------							
21	-	(Water @ EOB 5.0', 24hrs 5.0', 72hrs 5.0')							
	-								
22	-								
	-								
23	-								
	-								
24	-								
	-								
25	-								

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB10
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/17/2009
Elevation: 1091.54

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	10" of Dark Brown Silty SAND w/ Organics							
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	8	18	M		
	-	------ 10" ------							
2	-								
	-	Brown to Dark Brown Fine SAND, Little Silt							
3	-	(Probable Fill)							
	-	(USCS: SP-SM) (USDA: Loamy Sand)	2	3.5 - 5	4	18	M		
4	-	------ 4.5' ------							
	-	(Black Peat Layer 4.5' - 4.7')							
5	-	Brown F - M SAND							
	-	(USCS: SP) (USDA: Sand)							
6	-	------ 5.5' ------	3	6 - 7.5	5	18	S		
	-								
7	-	(Water @ 7.0', 24hrs - 6.9')							
	-								
8	-								
	-								
9	-	Light Brown Fine SAND	4	9 - 10.5	6	18	S		
	-	(USCS: SP) (USDA: Sand)							
10	-								
	-								
11	-								
	-								
12	-								
	-								
13	-	------ 13.0' ------							
	-								
14	-		5	14 - 15.5	4	18	S	1.0	
	-								
15	-								
	-								
16	-								
	-								
17	-								
	-								
18	-	Brown Sandy CLAY							
	-	(USCS: CH) (USDA: Clay Loam)							
19	-	(Probably Weathered From Granite)	6	19 - 20.5	8	18	S	2.0	
	-								
20	-								
	-								
21	-								
	-								
22	-								
	-								
23	-								
---	-		7	23.5 - 25	25	18	S		
25	-	------ E.O.B. 25.0' ------							
	-	------ Borehole Filled with Bentonite Chips ------							
	-	-- Water @ EOB 7.0', 24hrs 6.9' --							

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: Moved 21'E - See Plan
201 Reserve St, Stevens Point, WI

Boring: SB11
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/17/2009
Elevation: 1090.44

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Brown F - M SAND (Fill)							
1	-	(USCS: SP) (USDA: Sand)	1	1 - 2.5	7	18	M		
	-	------ 6" ------							
2	-	Brown F - M SAND, Some Silt (Fill)							
	-	(USCS: SM) (USDA: Loamy Sand)							
3	-	------ 3.0' ------							
	-	Brown to Black Silty SAND	2	3.5 - 5	4	18	M		
4	-	Trace Gravel, Few Wood Pieces							
	-	(Poss Fill) (USCS: SM) (USDA: Loamy Sand)							
5	-	------ 5.0' ------							
	-								
6	-	(Water@EOB 6.0', 24hrs - caved@5.6' & Dry)	3	6 - 7.5	6	18	S		
	-	Light Brown F - M SAND							
7	-	(USCS: SP) (USDA: Sand)							
	-								
8	-								
	-	------ 8.5' ------							
9	-		4	9 - 10.5	7	18	S		
	-								
10	-								
	-								
11	-								
	-								
12	-	Light Brown Fine SAND							
	-	(USCS: SP) (USDA: Sand)							
13	-								
	-								
14	-		5	14 - 15.5	12	18	S		
	-								
15	-								
	-								
16	-	------ 16.0' ------							
	-								
17	-								
	-								
18	-								
	-								
19	-		6	19 - 20.5	8	18	S	1.25	
	-	Brown/Green Sandy CLAY							
20	-	(USCS: CH) (USDA: Clay Loam)							
	-	(Probably Weathered From Granite)							
21	-								
	-								
22	-								
	-								
23	-								
---	-		7	23.5 - 25	21	18	S		
25	-	------ E.O.B. 25.0' ------							
	-	------ Borehole Filled with Bentonite Chips ------							
	-	-- Water @ EOB 6.0', 24hrs Dry (Cave-in 5.6') --							

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB12
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/17/2009
Elevation: 1089.39

Depth (ft)	Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
1	Dark Brown Silty SAND w/ Organics (Fill) (USCS: SM) (USDA: Sandy Loam) ------ 8" ------	1	1 - 2.5	4	18	M		
2	Black PEAT (USCS: Pt) (USDA: Muck)							
3	------ 3.0' ------	2	3.5 - 5	14	18	M		
4	Light Brown F - M SAND (USCS: SP) (USDA: Sand)							
5	(Water@EOB 6.0', 24hrs - caved@4.7' & Dry)							
6	------ 6.0' ------	3	6 - 7.5	6	18	S		
7								
8								
9		4	9 - 10.5	14	18	S		
10								
11	Light Brown Fine SAND (USCS: SP) (USDA: Sand)							
12								
13								
14		5	14 - 15.5	11	18	S		
15								
16								
17	------- 17.0' -------							
18	Brown GRAVEL, Some Sand (USCS: GP) (USDA: Gravelly Sand)	6	18.5 - 20	22	18	S		
19	(Probably Weathered From Granite)							
20	------ E.O.B. 20.0' ------							
21	------ Borehole Filled with Bentonite Chips ------ -- Water @ EOB 6.0', 24hrs Dry (Cave-in 4.7') --							
22								
23								
24								
25								

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB13
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/17/2009
Elevation: 1090.24

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
		Brown F - M SAND (Fill)							
1		(USCS: SP) (USDA: Sand)	1	1 - 2.5	21	18	M		Hit Cobble w/ Spoon
		------ 8" ------							
2		Brown to Dark Brown F - M SAND							
		Little Gravel, Cobbles (Fill)							
3		(USCS: SP) (USDA: Loamy Sand)							
		------ 3.0' ------	2	3.5 - 5	6	18	M		
4									
		Light Brown F - M SAND							
5		(USCS: SP) (USDA: Sand)							
6		------ 6.0' ------	3	6 - 7.5	6	18	S		
7									
8									
9			4	9 - 10.5	7	18	S		
10									
11									
12									
		Light Brown Fine SAND							
13		(USCS: SP) (USDA: Sand)							
14			5	14 - 15.5	3	18	S		
15									
16									
17									
18									
19			6	19 - 20.5	9	18	S		
20									
		------ 20.3' ------							
21									
		Gray/Green/Orange CLAY							
22		(USCS: CH) (USDA: Clay)							
		(Probable Weathered Granite Bedrock)							
23									
---			7	23.5 - 25	34	4	S		
25		------ E.O.B. 25.0' ------							
		------ Borehole Filled with Bentonite Chips ------							
		-- Water @ EOB 5.8', 24hr 5.8' --							

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: Moved 5'W - See Plan
201 Reserve St, Stevens Point, WI

Boring: SB14
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/18/2009
Elevation: 1089.34

Depth (ft)	Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
1	Dark Brown Silty SAND w/ Organics (Fill) (USCS: SM) (USDA: Sandy Loam) ------ 10" ------	1	1 - 2.5	3	4	M		
2	Black PEAT (USCS: Pt) (USDA: Muck) (Concrete@1.0') (Moved 5'W & Drilled Again)							
3	------ 3.0' ------	2	3.5 - 5	10	18	M		
4	Light Brown F - M SAND (USCS: SP) (USDA: Sand)							
5								
6	------ 6.0' ------	3	6 - 7.5	8	18	S		
7								
8								
9		4	9 - 10.5	5	18	S		
10								
11								
12	Light Brown Fine SAND (USCS: SP) (USDA: Sand)							
13								
14		5	14 - 15.5	5	18	S		
15								
16								
17								
18	------ 18.0' ------							
19	Gray CLAY, Some Sand, Some Gravel (USCS: CH) (USDA: Clay Loam) (Probably Weathered From Granite)	6	18.5 - 20	23	18	S	1.5	
20	------ E.O.B. 20.0' ------ ------ Borehole Filled with Bentonite Chips ------							
21	--Water @ EOB 5.5', 24hr Dry (Cave-in 4.9') --							
22								
23								
24								
25								

Nummelin Testing Services, Inc. **NTS #** **142.80**

SOIL BORING LOG

Boring By: Nummelin Testing Services, Inc.

Project: UWSP Hyer Hall
Location: See Plan
201 Reserve St, Stevens Point, WI

Boring: SB15
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/18/2009
Elevation: 1088.19

Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
	-	Dark Brown Silty SAND w/ Organics							
1	-	(USCS: SM) (USDA: Sandy Loam)	1	1 - 2.5	5	18	M		
	-	------ 8" ------							
2	-	Brown Fine SAND, Little Silt (Fill)							
	-	(USCS: SP-SM) (USDA: Loamy Sand)							
3	-	(Black Organic Silt 2.5' - 2.7')							
	-		2	3.5 - 5	9	18	S		
4	-	------ 4.0' ------							
	-								
5	-	Light Brown F - M SAND							
	-	(USCS: SP) (USDA: Sand)							
6	-	------ 6.0' ------	3	6 - 7.5	4	18	S		
	-								
7	-								
	-								
8	-								
	-								
9	-		4	9 - 10.5	4	18	S		
	-								
10	-								
	-								
11	-								
	-								
12	-								
	-								
13	-								
	-	Light Brown Fine SAND							
14	-	(USCS: SP) (USDA: Sand)	5	14 - 15.5	6	18	S		
	-								
15	-								
	-								
16	-								
	-								
17	-								
	-								
18	-								
	-								
19	-		6	19 - 20.5	12	18	S		
	-								
20	-								
	-								
21	-								
	-								
22	-	------ 23.0' ------							
	-	Brown/Red/Orange Sandy CLAY							
23	-	(USCS: CH) (USDA: Clay Loam)							
---	-	(Probably Weathered From Granite)	7	23.5 - 25	24	18	S		
25	-	------ E.O.B. 25.0' ------							
	-	------ Borehole Filled with Bentonite Chips ------							
	-	-- Water @ EOB 4.0', 24hr 4.0' --							

Nummelin Testing Services, Inc. **NTS #** **142.80**

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between $10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
1		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W
☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well:

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/16/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.): | Casing Diameter (in.):

Lower Drillhole Diameter (in.): | Casing Depth (ft.):

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)? | Depth to water (feet): 5.8

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal wt.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	6.0		

6. Comments
Borehole collapsed @ 6'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/17/09

Street or Route	Telephone Number
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By

Comments:

Signature of Person Doing Work: | Date Signed:

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
2		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W | ☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: _____

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/16/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	5.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal wt.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	5.0		

6. Comments
Borehole collapsed @ 5'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/17/09
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By
Comments	

Signature of Person Doing Work	Date Signed

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
3		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W | ☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: _____

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/17/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	5.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	5.0		

6. Comments
Borehole collapsed @ 5'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/18/09
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code	Signature of Person Doing Work	Date Signed
Stevens Point	WI	54481		

DNR Use Only

Date Received	Noted By
Comments	

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between $10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
4		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township N	Range ☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W | ☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well:

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/16/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.): | Casing Diameter (in.):

Lower Drillhole Diameter (in.): | Casing Depth (ft.):

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)? | Depth to water (feet): 6.2

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	6.6		

6. Comments
Borehole collapsed @ 6.6'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/17/09

Street or Route	Telephone Number
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By

Comments:

Signature of Person Doing Work: | Date Signed:

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
5		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W ☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: ____ ____ ____ ____

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City	State	ZIP Code

3. Well / Drillhole / Borehole Information

☐ Monitoring Well ☐ Water Well ☑ Borehole / Drillhole

Original Construction Date: 2/16/2009
If a Well Construction Report is available, please attach.

Construction Type: ☑ Drilled ☐ Driven (sandpoint) ☐ Dug ☐ Other (specify): _____

Formation Type: ☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.): | Casing Diameter (in.):

Lower Drillhole Diameter (in.): | Casing Depth (ft.):

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)? | Depth to water (feet): 6.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped ☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.) ☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry ☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout ☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	6.0		

6. Comments
Borehole collapsed @ 6'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/17/09

Street or Route	Telephone Number
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By

Comments:

Signature of Person Doing Work	Date Signed

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
6		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township N	Range ☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W ☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: _ _ _ _ _ _ _

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/16/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	6.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	5.3		

6. Comments
Borehole collapsed @ 5.3'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/17/09
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By
Comments	

Signature of Person Doing Work	Date Signed

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
7		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township N	Range ☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W | ☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: ____ ____ ____ ____ ____

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/16/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	6.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	6.2		

6. Comments
Borehole collapsed @ 6.2'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/17/09
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By
Comments	

Signature of Person Doing Work: | Date Signed:

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
8		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W
☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Latitude: DEG MIN SEC N
Longitude: DEG MIN SEC W

Reason For Abandonment:
WI Unique Well No. of Replacement Well: _ _ _ _ _ _ _ _ _ _

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/16/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.): | Casing Diameter (in.):

Lower Drillhole Diameter (in.): | Casing Depth (ft.):

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)? | Depth to water (feet): 6.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal wt.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	5.3		

6. Comments
Borehole collapsed @ 5.3'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/17/09
Street or Route: P.O. Box 127	Telephone Number: (715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

Signature of Person Doing Work: | Date Signed:

DNR Use Only

Date Received	Noted By
Comments	

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
10		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W ☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: ____ ____ ____ ____ ____

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/17/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	7.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	7.1		

6. Comments
Borehole collapsed @ 7.1'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/18/09
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By
Comments	

Signature of Person Doing Work: | Date Signed:

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between $10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
11		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township	Range
			N	☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W ☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Latitude: DEG MIN SEC N | Longitude: DEG MIN SEC W

Reason For Abandonment: | WI Unique Well No. of Replacement Well: ____ ____ ____ ____ ____

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City	State	ZIP Code

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/17/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.)	Casing Diameter (in.)
Lower Drillhole Diameter (in.)	**Casing Depth (ft.)**

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)?	Depth to water (feet)
	6.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	6.6		

6. Comments
Borehole collapsed @ 6.6'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/18/09
Street or Route	**Telephone Number**
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By
Comments	

Signature of Person Doing Work	Date Signed

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
Form 3300-005 (R 10/03) Page 1

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
12		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township N	Range E W

Grid Location: Feet ☐ N ☐ S ☐ E ☐ W
☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N
Longitude: DEG MIN SEC W

Reason For Abandonment:
WI Unique Well No. of Replacement Well: _ _ _ _ _ _ _

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/17/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.): | Casing Diameter (in.):

Lower Drillhole Diameter (in.): | Casing Depth (ft.):

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)? | Depth to water (feet): 6.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	4.8		

6. Comments
Borehole collapsed @ 4.8'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/18/09

Street or Route	Telephone Number
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By

Comments:

Signature of Person Doing Work: | Date Signed:

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

Well / Drillhole / Borehole Abandonment
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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
13		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township N	Range E W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W
☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N
Longitude: DEG MIN SEC W

Reason For Abandonment:
WI Unique Well No. of Replacement Well: ____ ____ ____ ____ ____

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/17/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.): | Casing Diameter (in.):

Lower Drillhole Diameter (in.): | Casing Depth (ft.):

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)? | Depth to water (feet): 5.8

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	6.0		

6. Comments
Borehole collapsed @ 6.0'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/18/09

Street or Route	Telephone Number
P.O. Box 127	(715) 341-7974

City	State	ZIP Code
Stevens Point	WI	54481

DNR Use Only

Date Received	Noted By

Comments:

Signature of Person Doing Work: | Date Signed:

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
14		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township N	Range E W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W
☐ Local Grid Origin ☐ (estimated) ☐ Well Location OR

Latitude: DEG MIN SEC N
Longitude: DEG MIN SEC W

Reason For Abandonment:
WI Unique Well No. of Replacement Well: _ _ _ _ _ _ _ _ _ _

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/18/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.): | Casing Diameter (in.):

Lower Drillhole Diameter (in.): | Casing Depth (ft.):

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)? | Depth to water (feet): 5.5

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	4.9		

6. Comments
Borehole collapsed @ 4.9'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment
NTS, Inc.	02/19/09
Street or Route: P.O. Box 127	Telephone Number: (715) 341-7974

City	State	ZIP Code	Signature of Person Doing Work	Date Signed
Stevens Point	WI	54481		

DNR Use Only

Date Received	Noted By
Comments	

State of Wisconsin- Dept of Natural Resources
P.O. Box 7921, Madison WI 53707-7921

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Route To:
☐ Drinking Water ☐ Watershed Water ☐ Waste Management ☐ Remediation/Redevelopment ☐ Other: _____

1. General Information

Boring Number	DNR Well ID No.	County
15		Portage

Common Well Name: | Gov't Lot # (if applic.):

1/4 / 1/4	1/4	Section	Township N	Range ☐ E ☐ W

Grid Location: Feet ☐ N ☐ E ☐ S ☐ W
☐ Local Grid Origin ☐ (estimated) OR ☐ Well Location

Latitude: DEG MIN SEC N
Longitude: DEG MIN SEC W

Reason For Abandonment:
WI Unique Well No. of Replacement Well: ___ ___ ___ ___ ___

2. Facility / Owner Information

Facility Name: UWSP Hyer Hall

Facility ID	License/Permit No.	City, Village, or Town
142.80		Stevens Point

Street Address of Well: 201 Reserve St

Present Well Owner: | Original Well Owner:

Street Address or Route of Owner:

City: | State: | ZIP Code:

3. Well / Drillhole / Borehole Information

☐ Monitoring Well
☐ Water Well
☑ Borehole / Drillhole

Original Construction Date: 2/18/2009
If a Well Construction Report is available, please attach.

Construction Type:
☑ Drilled ☐ Driven (sandpoint) ☐ Dug
☐ Other (specify): _____

Formation Type:
☑ Unconsolidated Formation ☐ Bedrock

Total Well Depth From Groundsurface (ft.): | Casing Diameter (in.):

Lower Drillhole Diameter (in.): | Casing Depth (ft.):

Was Well Annular Space Grouted? ☐ Yes ☐ No ☐ Unknown

If yes, to what depth (feet)? | Depth to water (feet): 4.0

4. Pump, Liner, Screen, Casing & Sealing Material

Pump and piping removed?	☐ Yes	☐ No	☑ N/A
Liner(s) removed?	☐ Yes	☐ No	☑ N/A
Screen removed?	☐ Yes	☐ No	☑ N/A
Casing left in place?	☐ Yes	☐ No	☑ N/A
Casing cut off below surface?	☐ Yes	☐ No	☑ N/A
Sealing material rise to surface?	☑ Yes	☐ No	☐ N/A
Material settle after 24 hrs?	☐ Yes	☐ No	☑ N/A
If yes, was hole retopped?	☐ Yes	☐ No	☑ N/A
If bentonite chips were used, were they hydrated with water from a known safe source?	☐ Yes	☐ No	☑ N/A

Required Method of Placing Sealing Material
☐ Conductor Pipe-Gravity ☐ Conductor Pipe-Pumped
☐ Screened and Poured (Bentonite Chips) ☐ Other (explain): _____

Sealing Materials
☐ Neat Cement Grout ☐ Clay Sand Slurry (11lb/gal w t.)
☐ Sand Cement (concrete) Grout ☐ Bentonite-Sand Slurry
☐ Concrete ☑ Bentonite Chips

For Monitoring Wells and Monitoring Well Boreholes Only:
☐ Bentonite Chips ☐ Bentonite-Cement Grout
☐ Granular Bentonite ☐ Bentonite-Sand Slurry

5. Material Used to Fill Well / Drillhole	From (ft.)	To (ft.)	No. Yards, Sacks Sealant or Volume (circle one)	Mix Ratio or Mud Weight
3/8" Bentonite Chips	Surface	4.2		

6. Comments
Borehole collapsed @ 4.2'.

7. Supervision of Work

Name of Person or Firm Doing Sealing Work	Date of Abandonment	City	State	ZIP Code
NTS, Inc.	02/19/09	Stevens Point	WI	54481

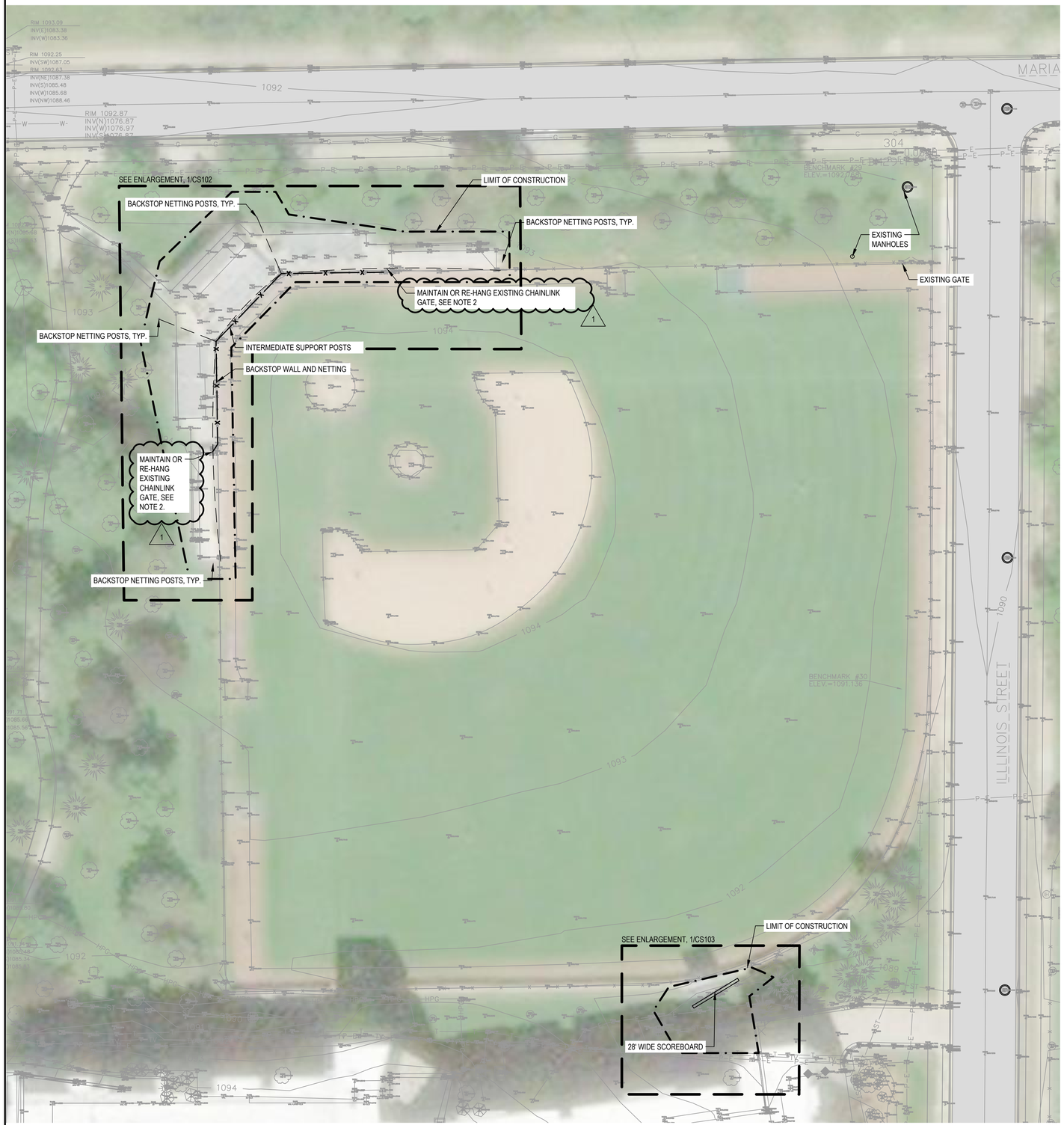
Street or Route: P.O. Box 127 | Telephone Number: (715) 341-7974

Signature of Person Doing Work: | Date Signed:

DNR Use Only

Date Received: | Noted By:

Comments:



1 BASEBALL FIELD PLAN VIEW

SCALE: 1" = 30'

1

GENERAL CONSTRUCTION -1-

1. W.H.C. SPECIFICATIONS

Wherever W.H.C. appears in this specification it refers to the Standard Specification of Road and Bridge Construction, Edition 1963 of the State Highway Commission of Wisconsin. EXCEPTION: This contract shall be a lump sum contract and the method of measurement and the basis of unit price payment according to W.H.C. does not apply.

2. EARTHWORK

Shall conform to W.H.C. Sections 201, 204, 205, 207, 208, 211 and 213. Contractor shall furnish all fill material required. All grades shown on plans are finished grades.

3. CHAIN LINK FENCE

Shall conform to W.H.C. Section 616.

A. Materials

(1) Type - Cyclone Safeguard, Century, or approved equal.

(2) Height - 4 ft. and 20 ft. as shown on the plans.

(3) Fabric - Hot-dip galvanized chain link 2 inch mesh, 9 guage, ASTM A392-55T, Class II installed with barbs down. Aluminum coated. Class II chain link fabric conforming to ASTM A491-63T is acceptable. All fabric shall be placed on the diamond side of the fence posts.

(4) Top rail and intermediate rail - 1 5/8" O.D. pipe, 2.27 lbs./ft. or 1 5/8" x 1 1/4" roll form section.

(5) Line posts - 2 1/2" O.D. pipe 3.65 lbs./ft. or Junior "H" column 2.72 lbs./ft. spaced at not more than 10' O.C.

Posts for 20'-0" high fence shall be 4" O.D. pipe 9.1 lbs./ft.

(6) Gate posts - 3" O.D. pipe 5.79 lbs./ft.

(7) Terminal posts and corner posts - 3" O.D. pipe 5.79 lbs./ft. or 3 1/2" x 3 1/2" roll formed sections with integral fabric loops 5.14 lbs./ft.

(8) Tension wire - bottom tension wire shall be 7 gauge galvanized steel wire.

(9) Gate frames - 1 5/8" O.D. pipe 2.72 lbs./ft. Provide latch and center stop and keeper.

(10) Post settings - 4 ft. fence shall have posts set 3'-0" deep in concrete. Concrete shall be 4'-0" deep and 10" diameter. 20 ft. fence shall have posts set 4'-0" deep in concrete. Concrete shall be 5'-0" deep and 16" diameter.

65/68

2 EXISTING BASEBALL BACKSTOP FENCE SPECIFICATION INFORMATION

INCLUDED FOR REFERENCE ONLY

SMITHGROUP

44 EAST MIFFLIN STREET
SUITE 500
MADISON, WI 53703
608.251.1177
www.smithgroup.com

The Board of Regents of the University of Wisconsin on behalf of the University of Wisconsin – Stevens Point

University of Wisconsin – Stevens Point
Stevens Point, WI 54481

Baseball Field Renovations
University of Wisconsin
– Stevens Point

Sheet Title: OVERALL SITE PLAN

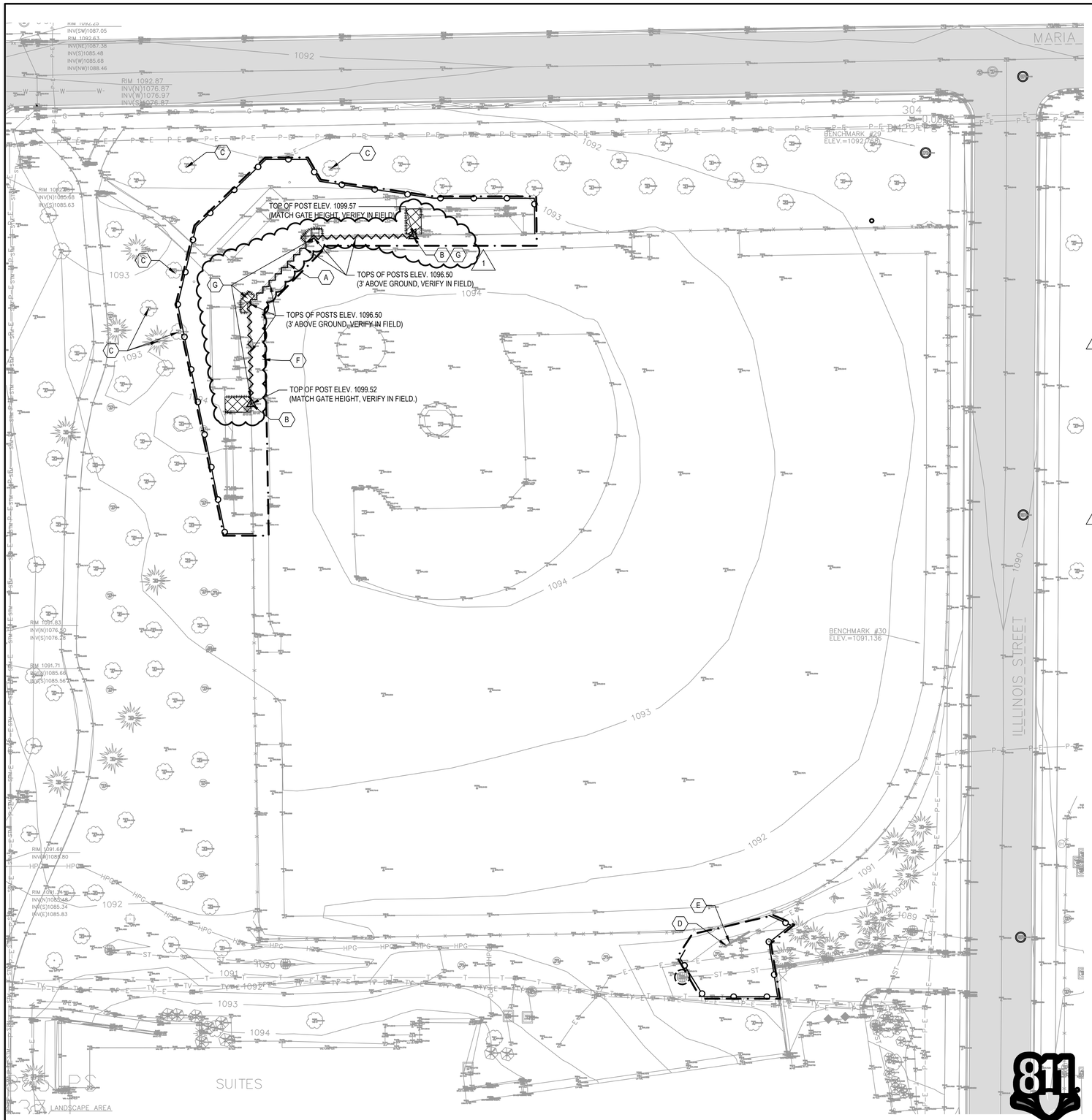
Revisions:

No.	Date:	Description:
1	21AUG19	ADDENDUM 1

Graphic Scale AS SHOWN

0' 15' 30' 60'

UWSA Number	K-19-001
Set Type	BD
Date Issued	07/30/2019
Sheet Number	CS100



Know what's **below.**
Call before you dig.

KEYED NOTES

A. EXISTING CHAIN LINK FENCE POSTS. CUT AND REMOVE POST ABOVE 3 FEET. REMOVE ALL CHAIN LINK INFILL. TREAT CUT GALVANIZED POST WITH A COLD GALVANIZING COMPOUND PRODUCT. REUSE EXISTING POST CAPS OR NEW CAP IF REQUIRED BY BACKSTOP NETTING PRODUCT.

B. EXISTING CHAIN LINK GATE AND POST. MATCH POSTS HEIGHT WITH GATE HEIGHT, CUT AND REMOVE PORTION ABOVE. TREAT CUT POST SIMILAR TO CUT FENCE POSTS AS STATED IN NOTE 'A'. REUSE EXISTING POST CAP (SEE NOT 'G' BELOW) 1

C. MINIMIZE DISTURBANCE AND COMPACTION ACTIVITIES AROUND EXISTING TREES. DISTURBANCE SHALL BE LIMITED TO ONLY THOSE AREAS NECESSARY TO COMPLETE SPECIFIED WORK.

D. EXISTING SCOREBOARD TO BE REMOVED IN IT'S ENTIRETY INCLUDING FOUNDATIONS, BY CONTRACTOR.

E. CAMPUS TO REMOVE AND TEMPORARILY SUPPORT THE IRRIGATION CONTROLLER. CAMPUS TO DISCONNECT ELECTRICAL AND COMMUNICATION CONNECTIONS AT SCOREBOARD. CAMPUS TO RECONNECT ELECTRICAL AND COMMUNICATION CONNECTIONS TO NEW SCOREBOARD. CAMPUS WILL REMOUNT IRRIGATION EQUIPMENT TO NEW SCOREBOARD POST. CONTRACTOR SHALL COORDINATE WORK WITH CAMPUS. CONTRACTOR SHALL TAKE PRECAUTIONS TO PROTECT UTILITIES.

F. MINIMIZE WORK ON FIELD SIDE OF BACKSTOP FENCE. ANY DISTURBANCE TO FIELD OR WARNING TRACK MUST BE RESTORED IN KIND AND BE IN OPERATIONAL CONDITION NO LESS THAN 2 WEEKS PRIOR TO FIRST BASEBALL GAME.

1 G. DESIGN INTENT IS TO MAINTAIN EXISTING BACKSTOP END AND CORNER POSTS (4) FOR REUSE. ORIGINAL SPEC OF BACKSTOP FENCE POSTS INCLUDED ON CS100. IF PROPOSED NEW BACKSTOPS REQUIRE NEW CORNER AND END POSTS, SAWCUT AND REMOVE CONCRETE PAVEMENT AND EXISTING POSTS. ONLY REMOVE EXTENT OF CONCRETE REQUIRED TO COMPLETE PROPOSED WORK. SAWCUT AT NEAREST JOINT IF PRACTICABLE.

LEGEND

- LIMIT OF WORK
- SALVAGE
- SEDIMENT CONTROL TUBE
- INLET PROTECTION
- 1 FENCE TO BE SHORTENED
- SAWCUT CONCRETE
- CONCRETE REMOVAL

SHEET NOTES

1
1. CONTRACTOR SHALL PROVIDE ALL EROSION CONTROL MEASURES AS NOTED ON THE DRAWINGS AND DEFINED IN SPECIFICATION 31 25 00, TO PROTECT PROPERTY AND THE ENVIRONMENT. APPLY AND PAY FOR EROSION CONTROL OR LAND DISTURBING PERMITS AS REQUIRED BY LOCAL MUNICIPALITIES AND STATE AGENCIES.
2. ALL DISTURBED AREAS SHALL BE RESTORED IN KIND. TURF AREAS SHALL BE RESTORED WITH SEED PER SECTION 32 92 19
3. CONTRACTOR IS RESPONSIBLE FOR ROUTINE SITE INSPECTIONS AT LEAST ONCE EVERY 7 DAYS AND WITHIN 24 HOURS AFTER A RAINFALL EVENT OF 0.5 INCHES OR GREATER. KEEP INSPECTION REPORTS ON-SITE AND MAKE THEM AVAILABLE UPON REQUEST.
4. INSPECT AND MAINTAIN ALL INSTALLED EROSION CONTROL PRACTICES UNTIL THE CONTRIBUTING DRAINAGE AREA HAS BEEN STABILIZED.
5. REFER TO THE WDNR STORMWATER CONSTRUCTION TECHNICAL STANDARDS AT http://dnr.wi.gov/topic/stormwater/standards/const_standards.html.
6. INSTALL PERIMETER EROSION CONTROLS PRIOR TO ANY LAND-DISTURBING ACTIVITIES, INCLUDING CLEARING AND GRUBBING.
7. STAGE CONSTRUCTION ACTIVITIES TO MINIMIZE THE CUMULATIVE EXPOSED AREA. CONDUCT TEMPORARY GRADING FOR EROSION CONTROL PER WDNR TECHNICAL STANDARD TEMPORARY GRADING PRACTICES FOR EROSION CONTROL #1067.
8. INSTALL AND MAINTAIN EROSION CONTROL TUBES PER WDNR TECHNICAL STANDARD. REMOVE SEDIMENT FROM BEHIND TUBES AND SEDIMENT BARRIERS BEFORE SEDIMENT REACHES A DEPTH THAT IS EQUAL TO ONE-HALF OF THE BARRIER HEIGHT. TUBES SHALL BE STABILIZED, MAINTAINED AND INSPECTED REGULARLY.
9. REPAIR BREAKS AND GAPS IN EROSION CONTROL TUBES AND BARRIERS IMMEDIATELY.
10. IMMEDIATELY STABILIZE STOCKPILES AND SURROUND STOCKPILES AS NEEDED WITH SILT FENCE OR OTHER PERIMETER CONTROL IF STOCKPILES WILL REMAIN INACTIVE FOR 7 DAYS OR LONGER.
11. IMMEDIATELY STABILIZE ALL DISTURBED AREAS THAT WILL REMAIN INACTIVE FOR 14 DAYS OR LONGER.
    - 11.1. BETWEEN SEPTEMBER 15 AND OCTOBER 15: STABILIZE WITH MULCH, TACKIFIER, AND A PERENNIAL SEED MIXED WITH WINTER WHEAT, ANNUAL OATS, OR ANNUAL RYE, AS APPROPRIATE FOR REGION AND SOIL TYPE.
    - 11.2. OCTOBER 15 THROUGH COLD WEATHER: STABILIZE WITH A POLYMER AND DORMANT SEED MIX, AS APPROPRIATE FOR REGION AND SOIL TYPE.
12. STABILIZE AREAS OF FINAL GRADING WITHIN 7 DAYS OF REACHING FINAL GRADE.
13. SWEEP/CLEAN UP ALL SEDIMENT/TRASH THAT MOVES OFF-SITE DUE TO CONSTRUCTION ACTIVITY OR STORM EVENTS BEFORE THE END OF THE SAME WORKDAY. SEPARATE SWEPT MATERIALS (SOILS AND TRASH) AND DISPOSE OF APPROPRIATELY.
14. CONTRACTOR IS RESPONSIBLE FOR CONTROLLING DUST PER WDNR TECHNICAL STANDARD DUST CONTROL ON CONSTRUCTION SITES # 1068.
15. PROPERLY DISPOSE OF ALL WASTE AND UNUSED BUILDING MATERIALS (INCLUDING GARBAGE, DEBRIS, CLEANING WASTES, OR OTHER CONSTRUCTION MATERIALS) AND DO NOT ALLOW THESE MATERIALS TO BE CARRIED BY RUNOFF INTO THE RECEIVING CHANNEL.
16. FOR NON-CHANNELIZED FLOW ON DISTURBED OR CONSTRUCTED SLOPES, PROVIDE APPROPRIATE (DEPENDING ON THE SLOPE) BIODEGRADABLE EROSION CONTROL MATTING. SELECT EROSION MATTING FROM APPROPRIATE MATRIX IN WDOT'S WIDOT PRODUCT ACCEPTABILITY LIST (PAL); INSTALL AND MAINTAIN PER WDNR TECHNICAL STANDARD NON-CHANNEL EROSION MAT #1052.
17. FOR CHANNELIZED FLOW ON DISTURBED OR CONSTRUCTED AREAS, PROVIDE APPROPRIATE (BASED ON SHEAR STRESS) BIODEGRADABLE EROSION CONTROL MATTING. SELECT EROSION MATTING FROM APPROPRIATE MATRIX IN WDOT'S WIDOT PRODUCT ACCEPTABILITY LIST (PAL); INSTALL AND MAINTAIN PER WDNR TECHNICAL STANDARD CHANNEL EROSION MAT #1053.
18. MAKE PROVISIONS FOR WATERING DURING THE FIRST 8 WEEKS FOLLOWING SEEDING OR PLANTING OF DISTURBED AREAS WHENEVER MORE THAN 7 CONSECUTIVE DAYS OF DRY WEATHER OCCUR.
19. INSTALL ADDITIONAL EROSION AND SEDIMENT CONTROL MEASURES AS NEEDED (SUCH AS TEMPORARY SEDIMENT BASINS, DITCH CHECKS, EROSION CONTROL MATTING, SILT FENCING, FILTER SOCKS, WATTLES, SWALES, ETC.).
20. CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE WDNR REMEDIATION AND WASTE MANAGEMENT REQUIREMENTS FOR HANDLING AND DISPOSING OF CONTAMINATED MATERIALS. SITE-SPECIFIC INFORMATION FOR AREAS WITH KNOWN OR SUSPECTED SOIL AND/OR GROUNDWATER CONTAMINATION CAN BE FOUND ON WDNR'S BUREAU OF REMEDIATION AND REDEVELOPMENT TRACKING SYSTEM (BRRTS) PUBLIC DATABASE AT: http://dnr.wi.gov/botw/

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The Board of Regents of the University of Wisconsin on behalf of the University of Wisconsin – Stevens Point

University of Wisconsin – Stevens Point
Stevens Point, WI 54481

Baseball Field Renovations University of Wisconsin – Stevens Point

Sheet Title: DEMOLITION AND EROSION CONTROL PLAN

Revisions:

No.	Date:	Description:
1	21AUG19	ADDENDUM 1

Graphic Scale 1" = 30'

0' 15' 30' 60'

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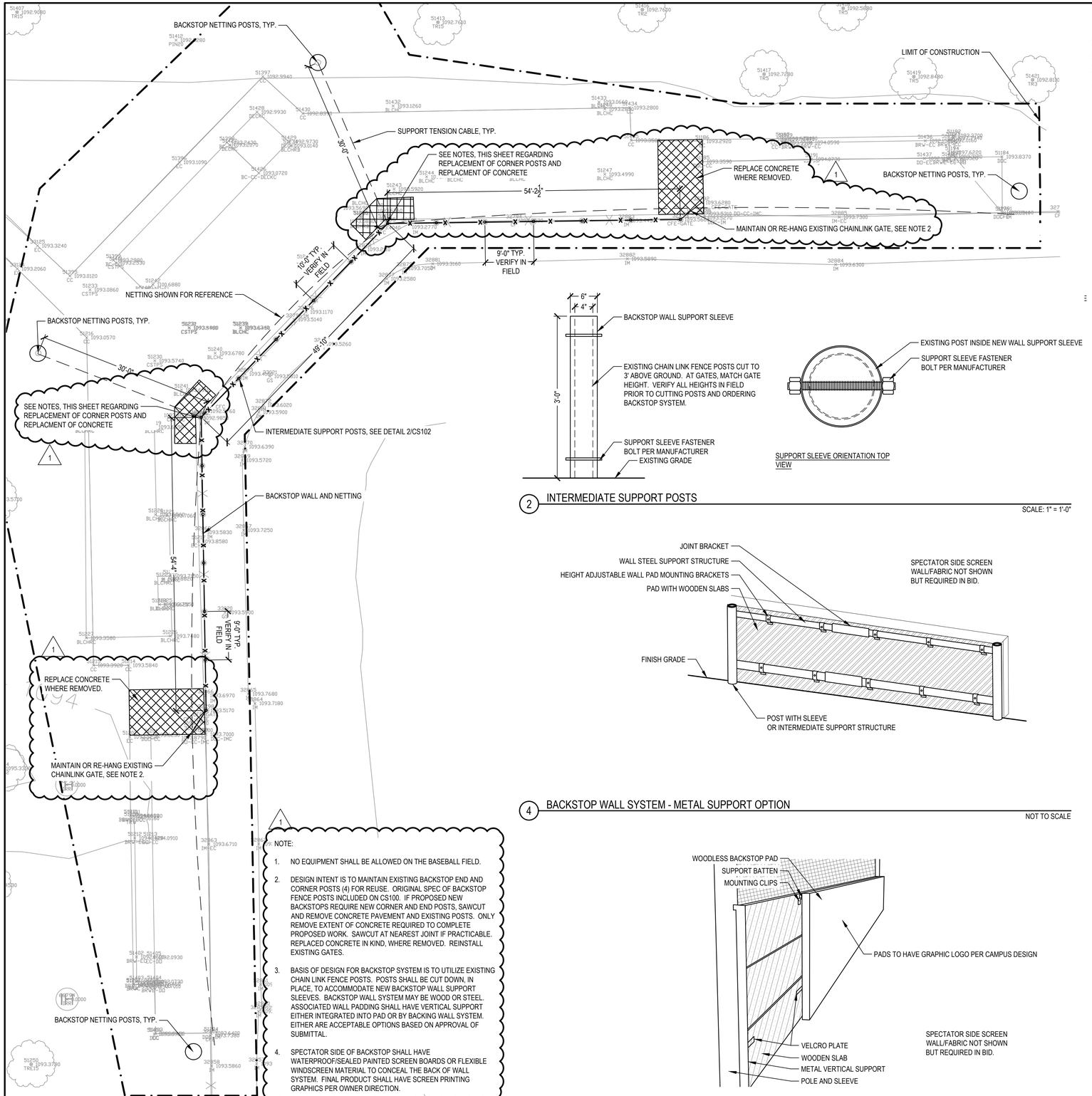


1 BACKSTOP WALL SYSTEM PLAN VIEW

SCALE: 1" = 10'

NOTE:

1. NO EQUIPMENT SHALL BE ALLOWED ON THE BASEBALL FIELD.
2. DESIGN INTENT IS TO MAINTAIN EXISTING BACKSTOP END AND CORNER POSTS (4) FOR REUSE. ORIGINAL SPEC OF BACKSTOP FENCE POSTS INCLUDED ON CS100. IF PROPOSED NEW BACKSTOPS REQUIRE NEW CORNER AND END POSTS, SAWCUT AND REMOVE CONCRETE PAVEMENT AND EXISTING POSTS. ONLY REMOVE EXTENT OF CONCRETE REQUIRED TO COMPLETE PROPOSED WORK. SAWCUT AT NEAREST JOINT IF PRACTICABLE. REPLACED CONCRETE IN KIND, WHERE REMOVED. REINSTALL EXISTING GATES.
3. BASIS OF DESIGN FOR BACKSTOP SYSTEM IS TO UTILIZE EXISTING CHAIN LINK FENCE POSTS. POSTS SHALL BE CUT DOWN, IN PLACE, TO ACCOMMODATE NEW BACKSTOP WALL SUPPORT SLEEVES. BACKSTOP WALL SYSTEM MAY BE WOOD OR STEEL. ASSOCIATED WALL PADDING SHALL HAVE VERTICAL SUPPORT EITHER INTEGRATED INTO PAD OR BY BACKING WALL SYSTEM. EITHER ARE ACCEPTABLE OPTIONS BASED ON APPROVAL OF SUBMITTAL.
4. SPECTATOR SIDE OF BACKSTOP SHALL HAVE WATERPROOF/SEALED PAINTED SCREEN BOARDS OR FLEXIBLE WINDSCREEN MATERIAL TO CONCEAL THE BACK OF WALL SYSTEM. FINAL PRODUCT SHALL HAVE SCREEN PRINTING GRAPHICS PER OWNER DIRECTION.

SUPPORT SLEEVE ORIENTATION TOP VIEW

2 INTERMEDIATE SUPPORT POSTS

SCALE: 1" = 1'-0"

4 BACKSTOP WALL SYSTEM - METAL SUPPORT OPTION

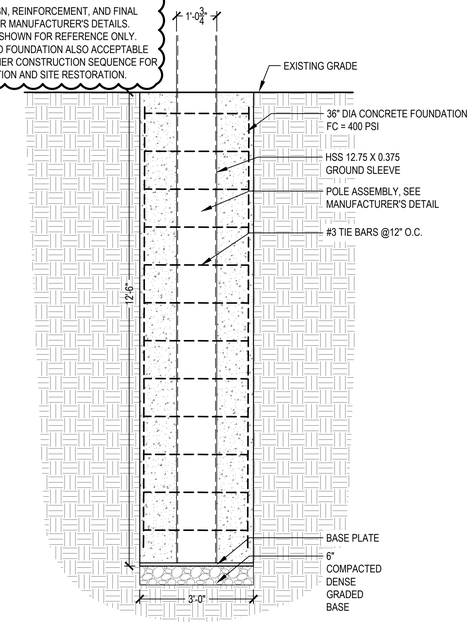
NOT TO SCALE

5 BACKSTOP WALL SYSTEM - WOOD WALL SUPPORT OPTION

NOT TO SCALE

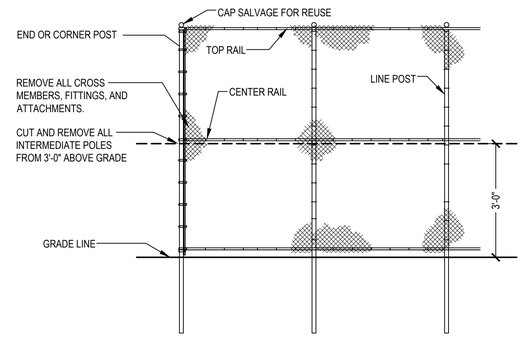
NOTE:

1. FINAL FOUNDATION DESIGN, REINFORCEMENT, AND FINAL DIMENSIONS SHALL BE PER MANUFACTURER'S DETAILS.
2. EMBEDDED FOUNDATION SHOWN FOR REFERENCE ONLY. SURFACE MOUNT POLE TO FOUNDATION ALSO ACCEPTABLE AND MAY PROMOTE EARLIER CONSTRUCTION SEQUENCE FOR FOUNDATION CONSTRUCTION AND SITE RESTORATION.

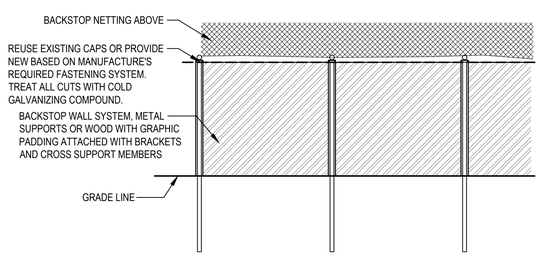


3 BACKSTOP NETTING POSTS

SCALE: 1/2" = 1'-0"



EXISTING CHAIN LINK FENCE



CUT INTERMEDIATE POLES FOR NEW BACKSTOP WALL SYSTEM

6 BACKSTOP WALL SYSTEM

SCALE: 1/2" = 1'-0"

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The Board of Regents of the University of Wisconsin on behalf of the University of Wisconsin – Stevens Point

University of Wisconsin – Stevens Point
Stevens Point, WI 54481

Baseball Field Renovations
University of Wisconsin – Stevens Point

Sheet Title: LAYOUT PLAN ENLARGEMENT AND DETAILS

Revisions:

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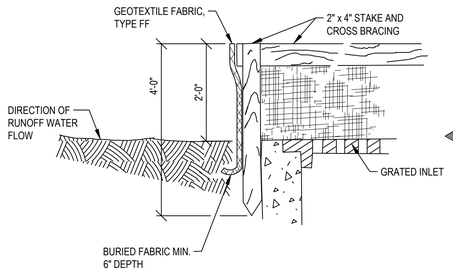
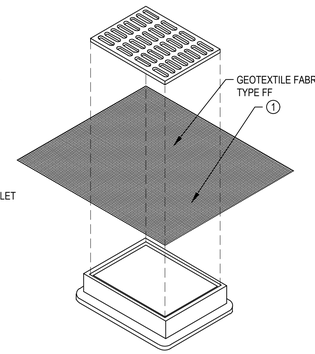
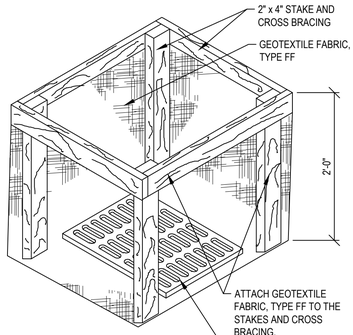


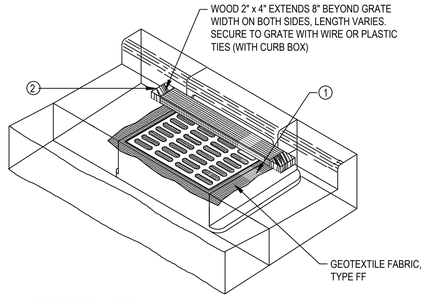


INLET PROTECTION, TYPE B
(WITHOUT CURB BOX)

(CAN BE INSTALLED IN ANY INLET WITHOUT A CURB BOX)



INLET PROTECTION, TYPE A



INLET PROTECTION, TYPE C

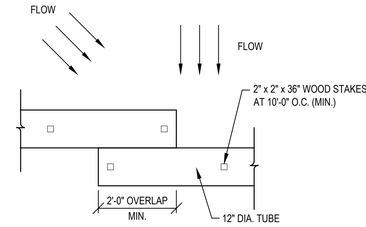
GENERAL NOTES:

MANUFACTURED ALTERNATIVES APPROVED AND LISTED ON THE DEPARTMENT'S EROSION CONTROL PRODUCT ACCEPTABILITY LIST MAY BE SUBSTITUTED. WHEN REMOVING OR MAINTAINING INLET PROTECTION, CARE SHALL BE TAKEN SO THAT THE SEDIMENT TRAPPED ON THE GEOTEXTILE FABRIC DOES NOT FALL INTO THE INLET. ANY MATERIAL FALLING INTO THE INLET SHALL BE REMOVED IMMEDIATELY.

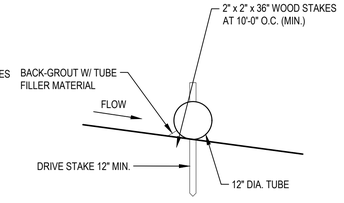
1. FINISHED SIZE, INCLUDING FLAP POCKETS WHERE REQUIRED, SHALL EXTEND A MINIMUM OF 10" AROUND THE PERIMETER TO FACILITATE MAINTENANCE OR REMOVAL.
2. FOR INLET PROTECTION, TYPE C (WITH CURB BOX), AN ADDITIONAL 18" OF FABRIC IS WRAPPED AROUND THE WOOD AND SECURED WITH STAPLES. THE WOOD SHALL NOT BLOCK THE ENTIRE HEIGHT OF THE CURB BOX OPENING.
3. FLAP POCKETS SHALL BE LARGE ENOUGH TO ACCEPT WOOD 2x4.

1 INLET PROTECTION

NOT TO SCALE



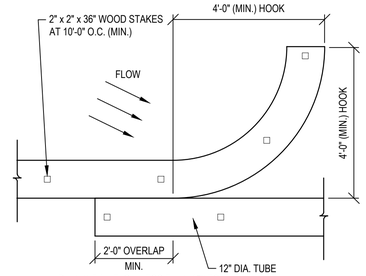
PLAN VIEW - FLOW 45 TO 90° TO TUBE



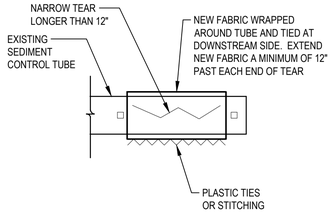
SECTION VIEW

NOTES:

1. WHEN SEDIMENT CONTROL TUBE IS USED ON A PAVED SURFACE CONCRETE BLOCKS SHALL BE USED TO SECURE SEDIMENT CONTROL TUBE IN PLACE OF STAKING.
2. ACCUMULATED SEDIMENT SHALL BE REMOVED FROM BEHIND THE SEDIMENT CONTROL TUBE WHEN IT HAS REACHED A HEIGHT EQUAL TO HALF OF THE TUBE HEIGHT.
3. SMALL HOLES OR NARROW RIPS LESS THAN 12" LONG MAY BE STITCHED CLOSED USING PLASTIC ZIP TIES. LARGER RIPS SHALL BE FIXED PER THE REPAIR DETAIL. HEAVILY DAMAGED SECTIONS SHALL BE REPLACED ENTIRELY. MAINTAIN 2' MINIMUM OVERLAP AT EACH END.

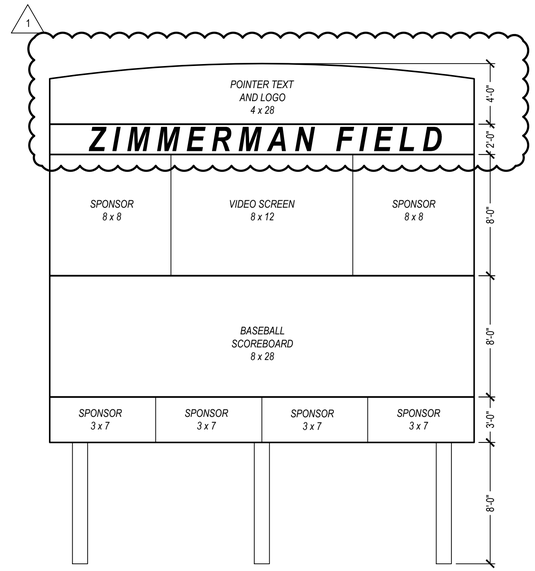


PLAN VIEW - FLOW LESS THAN 45° TO TUBE



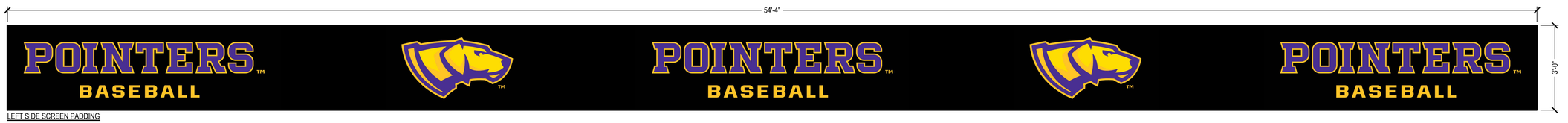
2 SEDIMENT CONTROL TUBE

NOT TO SCALE

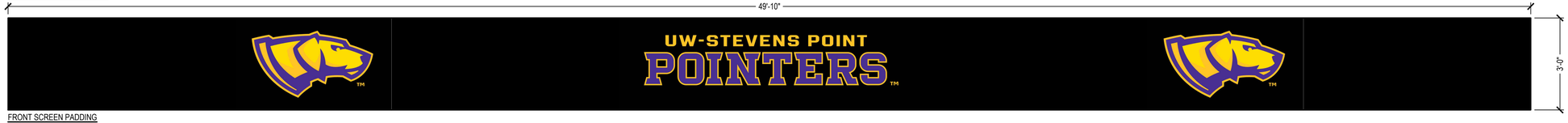


2 SCOREBOARD MOCKUP

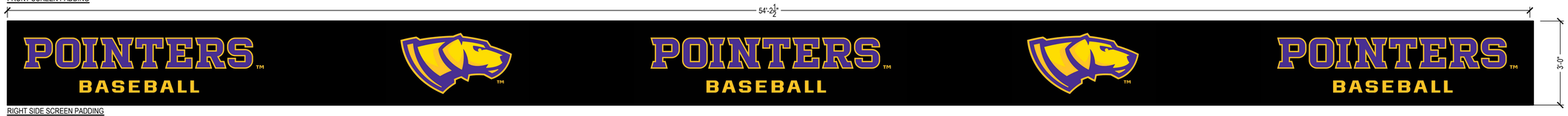
NOT TO SCALE



LEFT SIDE SCREEN PADDING



FRONT SCREEN PADDING



RIGHT SIDE SCREEN PADDING

3 BACKSTOP WALL PADDING GRAPHICS

NOT SHOWN: SPECTATOR SIDE SCREEN REQUIRED FOR BID AND INSTALLATION. SCREEN MAY BE FABRIC OR SEALED PAINTED WOOD. SCREEN SHALL HAVE GRAPHICS PRINTED PER CAMPUS DESIGN - PROVIDED DURING CONSTURCTION

1/2" = 1'-0"

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Sheet Title: SITE DETAILS - SITE PREPARATION

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