1		DENDUM N			
2	ISS	SUE DATE: A	august 22, 2019		
3					
4					
5	RE	: BASEB	ALL FIELD RE	NOVATIONS	
6		UNIVE	RSITY OF WISO	CONSIN – STEVENS POINT	
7		STEVE	NS POINT, WIS	SCONSIN	
8		UWSA I	Project No. <b>K-19</b> -	-001	
9					
10	BII	D OPENING:	GPC Bidders:	2:00 P.M., August 29, 2019	
11					
12	FR	OM:	SmithGroup		
13			44 East Mifflin	n Street, Suite 500	
14			Madison, Wisc		
15					
16					
17	TO	: Prospective	e Bidders		
18		•			
19	Thi	is addendum f	orms a part of th	he Bid Documents and modifies the original Bid Docum	ents dated July 30
20	201	19 as noted be	low. Acknowled	dge receipt of this Addendum by inserting the number an	id issue date of this
21	ado	dendum in the	blank space pr	rovided on the Bid Form. Failure to do so may sub	ject the Bidder to
22	dis	qualification.			
23					
24	Th	is Addendum	consists of (1)	one page and the attached documents, Specification S	ection 02 32 00 -
25	Ge	otechnical Inv	estigation, and f	full-size Drawings CS100, CS101, CS102 and CS500.	
26					
27	CH	IANGES TO S	ECTION 11 68 3	33 – ATHLETIC EQUIPMENT	
28	1.	Page 3, line	48, ADD: "Whe	ere Hot Dipped Galvanized Assembly Hardware is iden	tified, use of Zinc-
29		plated hardward	are will also be ac	cceptable."	
30		-		•	
31	2.	Page 4, line 6	, ADD: "2. Top:	: 2' x 28' Text "ZIMMERMAN FIELD"	
32					
33	CH	IANGES TO D	RAWINGS:		
34					
35	3.	Drawing CS	100 – OVERAL	LL SITE PLAN: Delete Drawing CS100 and replace w	ith revised CS100
36		attached.		•	
37					
38	4.	Drawing CS	101 – DEMOLI	ITION AND EROSION CONTROL PLAN: Delete Di	rawing CS101 and
39		replace with	revised CS101, at	ttached.	•
40		-			
41	5.	Drawing CS	102 – LAYOUT	Γ PLAN ENLARGEMENT AND DETAILS: Delete D	rawing CS102 and
42			revised CS102, at		-
43		•	ŕ		
44	6.	Drawing CS:	500 – SITE DET	TAILS - SITE PREPARATION: Delete Drawing CS50	0 and replace with
45		revised CS50	0, attached.		

END OF ADDENDUM NO. 1

46

47 48

1	CECTION 02 22 00							
1 2	SECTION 02 32 00 GEOTECHNICAL INVESTIGATION							
3	BASED ON DFD MASTER SPECIFICATION DATED 11/21/13							
4	BASED ON DED MASTER SPECIFICATION DATED 11/21/15							
5								
6	PART 1 - GENERAL							
7	TIMIT GENERAL							
8	SCOPE							
9	This section provides information resulting from subsurface investigations completed at the site as part of							
10	this project. This section may contain information applicable to ALL sitework, and other technical							
11	specification sections, as well. All Contractors are expected to review this information as part of their							
12	duties to familiarize themselves with the site.							
13								
14	Results of the geotechnical investigation apply only to the locations at which data was collected, at the							
15	specific time it was collected. Geotechnical conditions may differ elsewhere on the site.							
16								
17	Prior to making additional investigations of his own using test pits, borings, or other methods; Bidder shall							
18	first gain permission from property owner and UWSA Project Manager. Geotechnical investigations							
19	completed by Bidder shall comply with all applicable requirements of Division 01 through Division 33 of							
20	this project.							
21 22	RELATED WORK							
23	Applicable provisions of Division 01 govern work under this Section.							
24								
25	03 30 00 – Cast in Place Concrete							
26 27	11 68 33 – Athletic Equipment							
28	PART 2 - MATERIALS							
26 29	Not used.							
30	Not used.							
31	PART 3 - EXECUTION							
32	Not used.							
33								
34	END OF SECTION							



CENTRAL WISCONSIN AREA:
3217 Whiting Avenue
P.O. Box 127
Stevens Point, WI 54481 -0127
(715) 341-7974 • Fax (715) 341-8654

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### SUBSURFACE SOIL INVESTIGATION

PROPOSED UWSP RESIDENCE HALL
ILLINOIS AVENUE
STEVENS POINT
WISCONSIN

NTS 141.04

C08036

# PREPARED FOR:

ENGBERG ANDERSON
1 NORTH PINCKNEY STREET
P.O. BOX 100
MADISON, WI 53703-0100

ATTENTION: MR. JAMES BROWN



# FIELD INVESTIGATION BY:

NUMMELIN TESTING SERVICES, INC. 3217 WHITING AVENUE STEVENS POINT, WI

JULY 16, 2008

#### SUBSURFACE SOIL INVESTIGATION

# PROPOSED UWSP RESIDENCE HALL ILLINOIS AVENUE STEVENS POINT WISCONSIN

#### 1. INTRODUCTION

Nummelin Testing Services, Inc. (NTS) performed this investigation for the purpose of providing preliminary site information for the proposed University of Wisconsin – Stevens Point (UWSP) Residence Hall to be built on Illinois Avenue in the City of Stevens Point, Portage County, Wisconsin. The results and recommendations reported are based upon information obtained during a field investigation with borings, and the geotechnical analysis of that information. This report should be considered in its entirety during design and construction; no one section of the report should be used without considering all other sections.

The conclusions and recommendations reported are based on our interpretation of available subsurface and project information. The report may not represent variations that occur between or away from boring locations.

Should the scope of this project be altered, or if subsurface variations become evident during construction, it may be necessary to modify our recommendations. See the attached Geotechnical Engineering Report Information sheet for general information on NTS's geotechnical reports.

#### 2. PROJECT DESCRIPTION

The proposed project is the construction of a 4-story, dormitory-type, load-bearing masonry building with full basement on Illinois Avenue. The maximum interior wall bearing load is estimated at 30 kips, and the maximum exterior wall bearing load is estimated at 15 kips per lineal foot. The first floor elevation is expected to be near the existing grade.

At the time of the investigation, the surface of the site was covered by four tennis courts, three volleyball courts, and several clusters of trees. It is our understanding that this is a preliminary investigation, and another investigation will be performed at a later date.

#### 3. FIELD INVESTIGATION

Four (4) standard penetration soil borings were performed on July 7, 2008, with a truck-mounted, rotary drilling rig at locations shown on the attached sketch. Engberg Anderson determined the proposed boring locations and depths. At the time of drilling, NTS located the borings in the field. Boring 1 was drilled 18 feet south of the proposed location, and Boring 2 was drilled 8 feet west of the proposed location because trees blocked drill rig access to the proposed locations of Borings 1 and 2. Borings 3 and 4 were drilled at the proposed locations. Drilling between samples was by the hollow-stem-auger technique. Borings 1 and 2 were terminated at the proposed depth of 15 feet. Borings 3 and 4 were proposed to be drilled to a depth of 50 feet but were terminated because of auger refusal at depths of 32 and 19.5 feet, respectively.

Representative soil samples were obtained while boring using the Standard Penetration Test (SPT) method, in general accordance with ASTM Test Procedure D1586, at the depths indicated on the boring logs. The soils were visually classified in general accordance with ASTM Test Procedure D2488 by a technician at the time the borings were performed. The technician transferred the SPT samples to clean, glass jars immediately after retrieval and transported the samples to the NTS lab after completion of field operations. The soil samples have been examined in the laboratory by this writer and classified according to the USCS and, as requested, the USDA soil classification system. Both the USCS and USDA classifications appear on the logs. Soil parameters and classifications reported herein have been estimated based on the standard penetration test results, soil descriptions, and experience with soils in this area. No lab testing has been performed.

After completion of the borings, the bore holes were backfilled with bentonite chips to comply with WDNR requirements, and the last few inches were patched with auger cuttings.

At the time of the investigation, NTS determined ground elevations at the boring locations. The top of the large spout of the fire hydrant at the southeast corner of the site was used as a benchmark. According to the City of Stevens Point Engineering Department, the elevation of this benchmark is 1091.84.

Copies of the soil boring logs and a location sketch are appended to this report.

#### 4. SUBSURFACE CONDITIONS

# 4. 1. Area Geology

The subsoils in this area are mapped as outwash deposits. Outwash deposits generally consist of stratified sand and/or stratified sand and gravel. The underlying rock is mapped as granitic intrusive rock that is present at depths of less than 50 feet below the average surface terrain. A soil survey of Portage County maps the near-surface soils in this area as Roscommon muck that has been cut and filled at various locations. According to old soil maps, it appears that there used to be a creek near this area.

Note that mapped soil and bedrock conditions are provided for additional information only. Use of a design based on mapped or assumed conditions is not recommended.

# 4. 2. Soils at the Boring Locations

The general soil profile encountered at this site was silty sand fill and/or sedimentary peat from the surface to depths of 2 to 7 feet over very loose to medium-dense, native sand to depths of 13 to 17 feet over stiff clay (probably weathered from the underlying granite) to depths of 17 to 26.5 feet over granite bedrock, weathered to various degrees. The degree of weathering of the bedrock decreased with depth. A summary of the soil profile encountered in each boring is presented in Table 1.

Table 1. Soil Profiles in the Borings.

Boring #	Elevation	Sand Fill	Peat	Native Sand	Clay	Bedrock
1	1089.47	0'-3'	<u> </u>	3'-14'	14' – 15'+	_
2	1090.92	0'-3'	· _	3'-15'+	***	<u> </u>
3	1088.67	0'-6"	6"-2"	2'-17'	17' – 26.5'	26.5' – 32'+
4	1091.47	0'-3'	3'-7'	7' – 13'	13' – 17'	17' – 19.5'+

See individual boring logs for more detailed soil descriptions.

# 4. 3. Water Level Measurements

Water was encountered in all borings at depths of 3 to 6 feet (elevation 1085.12 to 1085.67). These water level measurements should be considered as representative of site conditions at the time of boring only. Expect seasonal fluctuations in the groundwater table of up to two feet.

# 5. DISCUSSION AND RECOMMENDATIONS

#### 5. 1. General

Plan to completely remove all existing pavement, any existing foundations, topsoil, and trees from the building footprint prior to further excavation and site grading.

The soils at this site, with no modifications, are expected to provide variable support for shallow foundations. Peat and fill occurred in the borings to depths of 2 to 7 feet, and, based on soil mapping of the area, the peat and fill likely occur elsewhere at this site, possibly to deeper depths. Undercutting to remove peat and/or fill from the base of shallow foundation excavations should be expected. The amount of undercutting will depend on the proposed depth of the foundations. The native sand encountered below the fill and peat was very loose to medium-dense. Compaction of the native sand will probably be necessary where the sand occurs at the base of excavations for shallow foundations.

Construction of a basement is not recommended at this site because, with water at depths of 3.0 to 6.0 feet (elevation 1085.12 to 1085.67), the potential for flooding of a belowground level is high. Should a below-ground level be constructed, it should be constructed at the highest elevation possible with an extensive underdrain system to prevent flooding.

Expect dewatering to be necessary for excavations near or below elevation 1085.7 at this site.

See below for further recommendations.

# 5. 2. Foundations and Floor Slab Support

Standard spread footings may be used at this site, but be aware that undercutting and/or compacting of existing soils will probably be necessary for shallow foundations. Deep foundations may also be used, but further exploration of the bedrock profile should be considered for deep foundations at this site because the bedrock depth and degree of weathering varied between borings. The depth to bedrock and degree of weathering have also been found to vary significantly over short distances in other areas around Stevens Point. Intermediate foundations (rammed aggregate piers) may also be considered.

A review of construction plans and records, if available, for nearby buildings may be beneficial for determining the most economical foundation type.

It is recommended that all load-bearing soils be inspected by NTS or another qualified soils engineering firm to verify that adequate bearing is present.

If the recommendations in this report are followed, total settlement should not exceed 1 inch, and differential settlement should not exceed one-half inch.

#### 5. 2. 1. Shallow Foundations

It is recommended that shallow foundations bear on the native sand or on fill meeting the requirements of section 5. 4. that bears on the native sand. Note that the native sand appeared very loose in places during boring, and undercutting/compaction of the native sand should be expected below shallow foundations. A presumptive allowable bearing capacity of 3,000 pounds per square foot (psf) may be used in the design of footings if the soil layer just below the footings is comprised of native sand and/or fill that meets these two requirements: the thickness of the layer must be at least one footing's width or 18 inches, whichever thickness is greater, and all soil in the layer must be compacted to at least 95% of the modified Proctor density (ASTM D-1557).

Shallow foundations should not bear on the peat or existing fill. Peat and/or fill were encountered in Borings 1 through 3 to depths of 2 to 3 feet and to a depth of 7 feet in Boring 4, and soil maps of the area suggest this peat and fill are likely to occur elsewhere at this site, possibly to deeper depths. Expect to undercut where peat and/or existing fill are found at the base of shallow foundation excavations.

# 5. 2. 2. Intermediate Foundations (Rammed Aggregate Piers)

NTS is not aware of rammed aggregate piers used for other buildings in Stevens Point, but sites with loose sands and high water table, such as this site, may be a good candidate for rammed aggregate piers. Rammed aggregate piers are constructed by ramming thin layers of aggregate into the ground with a high energy tamper. This forms a dense column of aggregate that offers support for bearing pressures of up to 10,000 psf. Rammed aggregate piers can also provide liquefaction mitigation. For more information regarding rammed aggregate piers, contact Steve Weyda of Geopier (262-628-1663).

# 5. 2. 3. Deep Foundations

Either piling or drilled shafts bearing on the less weathered bedrock at the terminal boring depths may be used for support, but further exploration of the bedrock profile should be considered for deep foundations at this site because the bedrock depth and degree of weathering varied between borings. The bedrock depth and degree of weathering have also been found to vary significantly over short distances in other areas around Stevens Point.

If piling is used, the piling should be driven to refusal into the bedrock. Maximum capacities for most driven piles are expected to be achieved in the bedrock at this site

near the depth of auger refusal in the borings, which varied from 32 feet in Boring 3 to 19.5 feet in Boring 4.

If caissons are used, caissons should be drilled to refusal in the bedrock. Because of the sandy soils and shallow water table at this site, drilled shafts should be cased and no belling should be considered. If casing is omitted or belling is used, the shaft will likely collapse, and adjacent locations will be undermined as soil migrates horizontally into the hole. A slurry replacement method may also be necessary for drilled shafts if soil and water seeps into the casing after the casing has been socketed into the rock. Recommended end bearing is 8 tons per square foot for design of caissons drilled to refusal in the bedrock. The base of each caisson should be thoroughly cleaned and, where possible, verified to be free of loose rubble/soils. Any verification of the caisson must follow OSHA regulations. The caisson drill must be capable of removing rock pieces of a diameter up to one-third of the diameter of the caisson. For a higher bearing pressure recommendation at a caisson base, a small diameter caisson load test may be performed. Loading to at least twice the design pressure value is recommended during the test. Caissons bearing on similar rock have been used at other buildings at the UWSP campus, such as the UWSP Fine Arts building. Construction records for these buildings may help in selecting a bearing value.

# 5. 2. 4. Floor Slab Support

The soils at this site are expected to provide variable support for a floor slab. After excavation for the floor, proof-roll the soils in the floor area. Proof-rolling will increase the density of the soils, providing more uniform support, and help to identify weak soils that are not suitable for support of the floor. An acceptable proof-roller for granular soil would be a smooth-drum, vibratory roller. Consider retaining NTS to observe the proof-rolling. Plan to undercut where weak soils are found. A subgrade modulus of 230 pounds per square inch per inch may be used for sand subgrades at this site which do not yield to the proof-roller.

Proof-rolling of soils near the water table at this site may cause the soils to become quick (liquefaction). The water table should be dropped at least 18 inches below the exposed grade prior to proof-rolling.

As an alternative to proof-rolling and undercutting, the floor slab may be structurally supported.

#### 5. 2. 5. Corrosion Potential

Any construction materials that will be placed in contact with organic soils should be protected against corrosion.

# 5. 3. Basement Drainage System

Construction of a basement is not recommended at this site because, with water at depths of 3.0 to 6.0 feet (elevation 1085.12 to 1085.67), the potential for flooding of a belowground level is high.

Should a below-ground level be constructed, it should be constructed at the highest elevation possible and an extensive underdrain system should be installed. Pumps for the underdrain system should be high-capacity pumps capable of running continuously, and a backup power source should be considered to run the pumps should the main power be lost. Drain tile for the system should be spaced at a frequency of no less than one drain per 15 feet, and all drain tiles should be part of one system. Several inches of sand conforming to ASTM C33 should be used as a filter around the drain tile and as the layer just beneath the floor. Native sand in the Stevens Point area is generally too fine and does not meet the ASTM C33 criteria. However, some native sand encountered at this site contained coarser material and may meet this requirement. A gradation test of sand proposed as filter sand would need to be performed to verify the sand's conformance. Clear stone or pea gravel does not work as a filter, and, although some codes require gravel for drains, neither stone nor gravel should be used for drains at this site. Also consider sealing the walls and floor to prevent moisture entering the basement through the walls and floor.

Regardless of actions taken, water problems should be expected at some point in the future in building levels at or below the water table.

# 5. 4. Compaction and Fill Requirements

The existing sand (both fill and native) encountered, if unsaturated and free of deleterious materials, is acceptable for use as structural fill. The peat encountered should not be used as structural fill. If imported fill is required, we recommend the fill be clean, unsaturated, granular soil. At the time of construction, NTS or another qualified soils engineering firm should verify that the proposed fill soils are acceptable. This firm should verify that the moisture content is appropriate for proper compaction and that the fill contains no deleterious materials. Frozen soil should not be used as structural fill.

Any required fill should be placed in lifts not exceeding 1 foot (uncompacted).

It is recommended that the water table be dropped prior to compaction because compaction of loose sand near or below the water table may cause the sand to become quick (liquefaction). If compaction of the sand near the water table is necessary, use light compaction equipment. To achieve required compaction levels with light equipment, lifts thinner than 1 foot may be necessary.

Compact all fill placed to at least 95% of the maximum dry density (modified Proctor method - ASTM D-1557). Site or soil conditions at the time of construction may warrant a change in the recommended compaction levels. However, no changes should be made without review by NTS or another qualified soils engineering firm.

#### 5. 5. Excavation

All excavations should comply with OSHA regulations. Excavation sidewalls in the onsite sands should have a slope no greater than 2.0 to 1.0. To minimize sidewall cave-in during construction, particularly during vibratory compaction, reduce the sidewall slope to approximately 30 degrees from the horizontal. Where steeper sidewall slopes are required or more convenient, temporary shoring will probably be necessary. Temporary shoring is generally left up to the contractor to design and construct.

Most common excavators (backhoes) should be adequate to make the necessary excavations.

Undercutting for non-structurally-supported floors and shallow foundations should be expected at this site. When undercutting, use the sixty-degree approximation to determine the resulting pressure at the base of the undercut. The recommended width of undercut is twice the undercut depth plus the width of the footing or floor. If the footing/floor location is accurately marked and centered in the base of the undercut, then the minimum width of the undercut is the depth of undercut below the footing plus the width of structure, measured at the base of the undercut. A good practice is to add at least one foot to this width. Replace all undercut soils with properly compacted fill (see section 5. 4. 'Compaction and Fill Requirements'). Compact any disturbed soil at the base of excavations/undercuts with a high energy compactor such as a hoe-pack or vibratory roller.

Excavations should be performed, where possible, using a flat plate attached to the bucket teeth of the backhoe to minimize disturbance at the base of the excavations. Where a toothed bucket is used, the last six inches or so should be excavated by turning the bucket so that the teeth are parallel to the proposed grade. Compact the base of all excavations.

Expect to encounter groundwater in excavations near and below elevation 1085.7. Dewatering will probably be required where water is encountered.

# 5. 5. 1. Excavation Dewatering

Dewatering will probably be necessary for excavations that extend below the water table. Where excavations are anticipated to extend below the water table, dewatering prior to excavation is recommended. When dewatering, avoid pumping water from

within the excavation. Pumping from within the excavation may loosen the soil at the base of the excavation as surrounding water flows into the excavation. This loosening may not be obvious during construction and can cause significant settlement of foundations. To avoid loosening soil at the base of the excavation, dewater with deep wells or well points from outside the excavation.

It is recommended that the water table be dropped at least 18 inches below the base of the excavation. Should the base of any excavation be at or slightly below the water table, roughly twelve inches of well graded, breaker-run rock (less than 10 percent P200 material) should be placed at the base of the excavation to provide a stable working platform and help prevent loosening of soils at the base of the excavation. The breaker-run should be placed and compacted immediately following excavation, and excavation before breaker-run placement should be limited to small areas.

# 5. 5. 2. Existing Structures

Care must be taken when excavating next to existing structures. Do not excavate soil under a line drawn out (away from existing structure) and down from the top of a footing at a 45-degree angle, unless proper precautions are taken. If excavations will extend below the elevation of the existing footings, the existing soil under the structure may have to be shored. This may be done using sheet piling, properly braced or tied back. Allow for imposed lateral loads from nearby footings in designing the system. Be aware that vibrations during driving of sheet piles may cause loose soils below the existing structure to settle. Monitor the existing structure for possible movement during the construction process. Construction records for the existing structure should be reviewed, if available. These may show engineered compacted granular fill below and outside existing footings. If this is the case, risk and severity of problems may be reduced if excavation below the elevation of the existing footing is required. Excavating small sections at a time may reduce the potential for seriously undermining the existing footing, and may allow construction to continue without the need to brace or use sheet piles.

Caution should be used when compacting adjacent to existing footings. Vibratory compacting equipment may cause some settling in adjacent soils below existing foundations. It may be better to use light compaction equipment near existing footings.

#### 5. 6. Lateral Earth Pressures

Unsaturated, firm, clean sand (unsaturated on-site sands and imported fill) will exert approximately 35 pounds per cubic foot (pcf) equivalent fluid pressure in the active state, approximately 330 pcf equivalent fluid pressure in the passive state, and approximately 55 pcf equivalent fluid pressure at rest.

Be aware that a saturated soil may exert up to three times as much pressure as a non-saturated soil, and successive passes of a compactor near earth-retaining structures may build up pressures significantly higher than those presented above.

# 5. 7. Coefficient of Static Friction for Sliding Analysis

For cast-in-place concrete against the sands at this site, a coefficient of static friction of not more than 0.577 should be used in conjunction with a safety factor against sliding of 1.5.

For pre-cast concrete against the sands at this site, a coefficient of static friction of not more than 0.462 should be used in conjunction with a safety factor against sliding of 1.5.

# 5. 8. Site Classification for Seismic Design

Most sand at this site is loose and below the water table, making the sand potentially liquefiable. According to Table 1615.1.1 of the 2002 Wisconsin Enrolled Commercial Building Code, the site classification for this site would be Site Class 'F' because of these potentially liquefiable soils.

# 5. 9. Pavement Design Considerations

The following section may be used in the design of any pavement at this site.

It is anticipated that the subgrade soils for the parking lot will consist of either the on-site sand fill or imported clean, fine sand fill. The on-site sand fill was very loose in some areas. It is recommended that the pavement subgrade be thoroughly compacted prior to base course placement.

The sand is slightly frost susceptible and has a Frost Group Designation of F-2. A CBR test was not performed. However, the CBR factor for this material based on the soil description is estimated to be 15.0. The Wisconsin Asphalt Pavement Association Design Guide suggests a soil support value of 5.0 for soils with a CBR value between 10 and 20, classifying the subgrade as 'good'. A subgrade modulus of not more than 230 pounds per square inch per inch should be used for pavement and slab design on these soils.

If asphaltic pavement is used, design thicknesses will vary with expected traffic loads. The traffic load refers to the expected design daily ESALs (18,000 pound Equivalent Single Axle Loads). The following suggested pavement and crushed aggregate base thicknesses are based on 1 to 5 design daily ESALs (usually residential streets, car parking lots, and similar traffic loads).

Subsurface Soil Investigation Report UWSP Residence Hall Illinois Avenue, Stevens Point, WI

5. 9. 1. Flexible Pavement, 1 to 5 Design Daily ESALs

Use a minimum of 8 inches of crushed aggregate base course with a minimum of 3 inches of asphaltic concrete.

If the expected daily ESALs are greater, plan to increase these thicknesses.

Rigid (Portland cement concrete) pavement should be used at locations of trash dumpsters or other heavy-duty traffic locations. A thickness of at least 8 inches is recommended for concrete slabs. The base course thickness below the slab is not critical, but it should be thick enough to provide stability for the slab.

The pavement construction should meet the requirements of the Wisconsin DOT Standard Specifications for Road and Bridge Construction.

A prime requirement for successful pavement is preparation of the subgrade soil. At the time the base course is being place, the subgrade should be firm when proof-rolled. Proof-rolling of the proposed pavement subgrade will help identify areas containing peat or other weak soils that may require undercutting. An acceptable proof-roller for sand would be a large, smooth-drum, vibratory compactor. The subgrade may yield slightly to the proof-roller, but after base course placement the base grade should be unyielding to the fully loaded, tandem-axle, dump trucks. This requirement also applies after the completion of any undercut.

It may be necessary to undercut and replace soft or loose soils with crushed rock or breaker-run rock. Any rock used to stabilize a soft subgrade should not be considered as part of the base course thickness.

Respectfully,

Bergamm K. Mumuelin

Clifton E.R. Lawson, P.E.

Consulting Engineer

Cliffer ER. lawson / Mu

Benjamin K. Nummelin, E.I.T. Nummelin Testing Services, Inc.

bkn/cerl/bn/mn

NTS 141.04

# NUMMELIN TESTING SERVICES, INC

# GEOTECHNICAL ENGINEERING REPORT INFORMATION SHEET

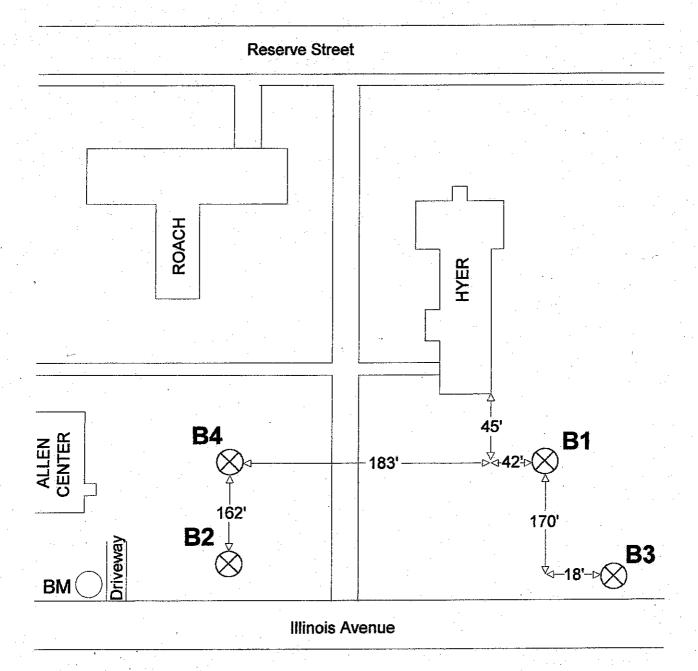
Ì	
	Subsurface soil conditions are responsible for many of the construction problems encountered at building sites. In order to help you, our client, manage your risks, we offer you the following information and suggestions.
Parallel Par	Geotechnical engineering reports are based on observations of specific soil conditions existing at the time of the subsurface soil investigation. As these conditions may change over time, construction decisions should be made with the timeliness of the report in mind. Further testing may be advisable if subsurface soil conditions are affected by natural events (flooding, spring thaws, etc.) and construction (drilling, blasting, surcharges, etc.) on-site or adjacent to it. Talking to your geotechnical professional before construction begins will help keep one informed if further tests are recommended.
Analysta majanta and a Managamini, philippin and a structure of the struct	The recommendations included in your geotechnical engineering report are based on a limited number of samples/tests. These recommendations assume that subsurface conditions throughout the site will be similar to those observed. As all recommendations are preliminary when based on limited testing, it is important to have your geotechnical professional observe the actual conditions during construction. This allows him/her to note any differences that may not have been revealed by the limited samples/tests and/or that are more abrupt than reported in the preliminary report. It is this geotechnical professional, using his/her knowledge and familiarity of site history, as well as construction observations, who will be able to determine if there is adequate and appropriate support to consider these recommendations final. He/she will also be able to document that the contractor is following these recommendations. Be aware that this geotechnical professional can not assume responsibility and/or liability for his/her recommendations based on observations and determinations by others.
Synthetical breathernames broaternames	Professional judgement, based on experience and observations, is at the heart of our geotechnical recommendations. Geotechnical reports use information from a limited number of samples/tests to predict conditions regarding your overall site. No one may say with certainty what subsurface conditions really exist without actual observation. The conditions away from sample/test areas may vary from what is predicted. It is important to identify variations as early as possible. This is why we encourage you to take advantage of our knowledge and experience during the construction phase of your project. Working together we can help minimize the impact when unexpected variations occur.
hospitamentational (constitutional and an arrangement and arrangement)	Geotechnical reports are written for a specific client, purpose, project and set of conditions. They are not intended to be a generalized, generic report for a proposed site. They are for the sole use of our client for the express purpose indicated to us. Should the scope of the project be altered, or if subsurface variations become evident during construction, it may be necessary to modify our recommendations. Early communication with your geotechnical professional can help you avoid expensive problems that may occur when changes to a project's purpose, structure, size, usage, site orientation, elevation, etc. are made after a report is written.
Contraction (Contraction)	Following these guidelines, your geotechnical subsurface report should provide informed and accurate information to assist in the planning and construction of your project

PROJECT: UWSP Residence CLIENT: Engberg Anderson

PROJECT #: 141.04 DATE: 07-10-08



 $N \rightarrow$ 



Drawing NOT to Scale Locations Approximate BY: BKN NUMMELIN TESTING SERVICES, INC. 3217 WHITING AVENUE STEVENS POINT, WI 54481 (716) 341-7974

#### **BORING LOG NOTES**

DESCRIPTIVE TERM, GRANULAR SOIL (% BY DRY WEIGHT)

Trace 0% - 5% Little 5% - 12%

Some 12% - 35%

And 35% - 50%

Q<sub>P</sub> = Estimated Unconfined Compressive Strength (by pocket penetrometer)

expressed in tons per square foot (t/sf).

 $Q_U$  = Estimated Unconfined Compressive Strength (by ASTM 2166)

expressed in tons per square foot (t/sf).

NM = Natural Moisture

M = MOISTURE

D = Dry F = FrozenM = Moist W = Wet

S = Saturated

LOI = Loss on Ignition (Organic Content)

N (Standard Blow Count) = blows per foot, as shown. Performed in general accordance with Standard Penetration Test Specifications (ASTM D-1586).

NR = No Recovery

WOH = Weight of Hammer

# = Sample Number

SOIL CLASSIFICATION

F = Fine

M = Medium

C = Coarse

LL = Liquid Limit, percent

PL = Plastic Limit, percent

PI = Plasticity Index (LL-PL)

W.L. = Water Level

#### SOIL STRENGTH CHARACTERISTICS

CONSISTENCY	(Cohesive Soils)	RELATIVE DENSITY (Granular Soil						
<u>Term</u>	Q <sub>11</sub> tons/sq.ft.	<u>Term</u>	"N" Value					
Very Soft	0.0 to 0.25	Very Loose	0 - 4					
Soft	0.25 to 0.50	Loose	4 - 10					
Firm	0.50 to 1.0	Medium-Dense	10 - 30					
Stiff	1.0 to 2.0	Dense	30 - 50					
Very Stiff	2.0 to 4.0	Very Dense	Over 50					
Hard	Over 4.0		•					

#### ORGANIC CONTENT BY COMBUSTION METHOD PLASTICITY

Soil Description	Loss On Ignition	<u>Term</u>	Plastic Index
Non Organic	Less than 4%	None to Slight	0 - 4
Organic Silt / Clay	4 - 12%	Slight	5 - 7
Sedimentary Peat	12 - 50%	Medium	8 - 22
Fibrous & Woody Peat	More than 50%	High to Very High	Over 22

bornotes.bor

Project:

Location:

Boring By: Nummelin Testing Services, Inc.

**UWSP Residence Hall** 

**Boring:** 

2 1/4" HSA

Auger:

1 of 1

Page: **Drillers:** 

NH / MA

Date:

7/7/08

Moved 18'S of Proposed Location b/c of Trees - See Plan

,			Illinois Avenue, Stevens Point, WI 54481	·				Elev	ation:	1089.47
İ	Depth		Classification/Description	#	Sample	N	Rec	M	Qp	Notes
]	(ft)				Depth(ft)		(in)		(tsf)	
		-	Dark Brown Silty Fine SAND w/ Organics	1	0' - 2'	30	10"	М		Cobble
ĺ	1	-	( USCS - SM, USDA - Sandy Silt )							
ί		_	4"							·
	2	-	Black Fine - Medium SAND, Clay Seams, Organics							
N. Carlo		_	Cobbles (Fill) (USCS - SP, USDA - Sand)							
]	3	- :	3.0'	'						
)		_		2	3.5' - 5'	9	18"	S		,
-	4	_				] _	"	~		
<b>ئ</b>		_								
	5	_				1				
- Aller Season A										
3	6			3	6' - 7.5'	8	18"	S		·
]	"	-			0 - 7.5	ľ	10			
restitition	7	_								
5	'	_								
NO.	8	-	Grayish Brown Fine - Medium SAND							
Shipadas	ľ	-	l. •							
		-	( USCS - SP, USDA - Sand )		01 10 51	۱.,	18"	ď		
proma	9	-		4	9' - 10.5'	11	18"	S		
- Accessor		-				1			٠	
	10	- i				İ	j j			· [
Manage		-							1.0	
**************************************	11	-			-					
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STATE OF THE PERSON NAMED IN		-								
	13	-								
, tampon tam tampon tam tam tam tam tam tam tam tam tam tam		, <b>-</b>		5	13.5' - 15'	4	10"	W		1.0
, Augusta	14	-	14.0'							
İ		-	Reddish Brown Sandy CLAY, Little Gravel				.			
Management	15	-	( Possibly Weathered From Granite )							.
, second		-	( USCS - CL, USDA - Clay Loam )		. ,					
	16	-	E.O.B. 15'							
Western W.		-	Water at Completion 4.3'							
	17	-	Cave-in @ 4.4'			٠.				
_		-	Bore Hole filled with Bentonite Chips					ļ		.~
Surgeon S	18	-				. ;	·			
		-								
$\lfloor \rfloor$	19	_								
and distanced		_								
1	20	-								

Project:

Location:

Boring By: Nummelin Testing Services, Inc.

**UWSP Residence** 

**Boring:** Auger:

2 1/4" HSA

1 of 1

Page: **Drillers:** 

Date:

NH/MA 7/7/08

Illinois Avenue, Stevens Point, WI 54481

Moved 8'W of Proposed Location b/c of Trees - See Plan

Elevation:

1090.92

1			Illinois Avenue, Stevens Point, WI 54481					Liev	ration:	1090.92
	Depth		Classification/Description	#	Sample	N	Rec	M	Qp.	Notes
	(ft)		•		Depth(ft)		(in)		(tsf)	
	(-)	-	Dark Brown Silty Fine SAND w/ Organics	1	0' - 2'	21	24"			
	. 1	_	(USCS - SM, USDA - Sandy Silt)							
J.		_	8 <sup>11</sup>							
	2		Brown Fine - Medium SAND, Little Gravel (Fill)							
	2	_ ;	(USCS - SP, USDA - Sand)							4 *
J	3	_	3.0'							
,	٠	-	J.O	2	3.5' - 5'	10	18"	М		
	4			_	. 3.3 - 3	10	10	141		
	4	_								
		-								
aleri de Garia	5	-								
	_	-		,	CI 7 EI	9	18"	w		·
7	6	-		3	6' - 7.5'	9	10	W		
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	7	-						:		
1		-						٠.		
	8	-,							•	
,š	_	-						~		
	9	- j	Grayish Brown Fine - Medium SAND	4	9' - 10.5'	4	18"	S		
New York		-	( USCS - SP, USDA - Sand )					:		
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Ole State	. ]	-								
	13	-							1	
Amount		-,		5	13.5' - 15'	8	18"	S	]	
a course	14	-							]	
	Ì	-								
	15									
to Modern	ļ	-	E.O.B. 15'							
	16	-	Water at Completion 5.8'	,					1	
1	İ	-	Cave-in @ 5.9'							}
Taxana I	17	_	Bore Hole filled with Bentonite Chips				,			
	l	-								
National Park	18	_								
	]	- ]								
	19	_								
		-								
	20	- 1								·
L							L	L		

Boring By: Nummelin Testing Services, Inc.

Boring:

3 1/4" HSA

Auger: Page:

1 of 2

Project: UWSP Residence Hall Drille

Drillers: NH / MA

Location: See Plan

Date:

7/7/08

Illinois Avenue, Stevens Point, WI 54481

**Elevation:** 1088.67

	D 4		Classification/Description	#	Sample	N	Rec	M	On	Notes
	Depth		Classification/Description	#		IA	ž į	. 141	Qp	110103
,	(ft)				Depth(ft)		(in)		(tsf)	
		_	Dark Brown Sandy SILT w/ Organics	1	0' - 2'	4	24	M		
	1	_	(USCS - ML, USDA - Sandy Silt)							
	•		6"							· •
	_	-	•							
I	2	-	Black Sed. PEAT ( USCS - Pt, USDA - Muck )							
1		-	2.0'							
-,	3	_								
٠,				2.	3.5' ~ 5'	8	18"	w		
	4	_			5.5	٠,	``	"		
	4	-								
		-						1		
	5	-			·					.
		_								
<i>-</i>	6			3	6' - 7.5'	- 2	18"	S		l
,	U	_			0 - 7,5		10			
	•	-				•				
	7	-								
		-		]		·				
1	8	_	Grayish Brown Fine - Medium SAND							
Feedowa	Ţ.,	_	(USCS - SP, USDA - Sand)							
	9	_	( OBCB - BI, OBDA - Bank )	4	9' - 10.5'	8	18	W		
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	14			5	14' - 15.5'	2	10"	W		
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		-				1		Ī		
	-15	-				l '				
		-						l		
-	16	- 1				1				
, [	-~ [	_ [	(Harder Drilling @ 17')							
	,,,	_								
	17	-	17.0'		. !					
		-								
	18	-	Reddish Brown Sandy CLAY, Little Gravel							
		_	( Possibly Weathered From Granite )							
ı	19	_	(USCS - CL, USDA - Clay Loam)	6	19' - 20.5'	8	10	W	2.25	
	17	_	( Obeb - Cb, ObbA - Clay Loani )		17 20.0		•	''	رسيس	
		-		•					-	
Ĺ	20	-							Pidde 4	أحييا

Boring By: Nummelin Testing Services, Inc.

Boring:

2 1/4" HSA

ng by: Mullimenn resung Services, inc

Auger: Page:

2 of 2

Project: UWSP Residence Hall Location: See Plan

Drillers:

NH / MA 7/7/2008

Project:			Ovvor Residence nam			Driners:			NEL / IVIA	
Location:			See Plan		•			Date		7/7/2008
		<u>,</u>	Illinois Avenue, Stevens Point, WI 54481						ation:	1088.67
	Depth	1	Classification/Description	#	Sample	N	Rec	M	Qp	Notes
]	(ft)	1			Depth(ft)		(in)		(tsf)	
	<del></del>	1 -					<u> </u>		· · · · · · · · · · · · · · · · · · ·	
	21	_						ļ		
	]	_								
	22	1 .		1 1					·	i i
	. 22	-	Dadish Darras Canda CLAN 1341, Cand							
The same of the sa	22	-	Reddish Brown Sandy CLAY, Little Gravel					•		·
	23	-	(Possibly Weathered From Granite)							
7		-	( USCS - CL, USDA - Clay Loam )							
-	24	-		7	24' - 25.5'	18	10	W		
	•	-								·
٠,	25	-								
prised to the contract to the		_ `						•		
	26		( Very Hard Drilling @ 26.5')						ľ	
			26.5'							
eversomoseve	.27	_								
Nac.										
	28									
**************************************	26	_	Worth and Carrite Dalas 1							·
i processor	20	-	Weathered Granite Bedrock		201 20 51	50 IO	100			
	29	-	( USCS - N/A, USDA - Weathered Rock )	8	29' - 30.5'	50/2	18"	M		1
Sympa		-		İ						
	30	-								
		- :								
Day.	31	-								į
	1	-	( Auger Refusal @ 32')							. 1
ı	32	-	E.O.B. 32'							
N. S		-	Water at Completion 3.0'	·						
igazatata Paratata	33	-	Cave-in @ 3.5'							
		- ]	Bore Hole filled with Bentonite Chips			•				
and a	34	_	*							
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e constant	36	_ }								
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8 -		:_ T							AUTO 4	

Project:

Location:

Boring By: Nummelin Testing Services, Inc.

See Plan

**UWSP Residence Hall** 

Boring:

Auger:

3 1/4" HSA 1 of 1

Page: **Drillers:** 

NH/MA

Date:

7/7/08

Illinois Avenue, Stevens Point, WI 54481

1091.47 **Elevation:** 

3		Illinois Avenue, Stevens Point, WI 54481			<u>.</u>		ESTON	ation:	1,091.47
Depth		Classification/Description	#	Sample	N	Rec	M	Qp	Notes
(ft)	:			Depth(ft)		(in)		(tsf)	
1	1	Dark Brown Silty Fine SAND w/ Organics	1	0' - 2'	14	24	M		
1 1	-	( USCS - SM, USDA - Sandy Silt )							
1	-	6"							
2	-	Brown Fine to Medium SAND, Some Gravel							
	-	(Fill) (USCS - SP, USDA - Sand)					-		
3	-	3.0' <b></b> -							
1	-		2	3.5' - 5'	2	18"	W		
4	-						٠		
	-	Black Sedimentary PEAT					·		
1 5	·	(USCS - Pt, USDA - Muck)							
6	_		3	6' - 7.5'	2	18"	S		
	_								
7	-	7.0'				·			·
8	_							,	· ·
	_				,				
9			4	9' - 10.5'	8	18	W		
- Alexandra	1	Grayish Brown Fine - Medium SAND							
10	_	( USCS - SP, USDA - Sand )							:
	_					٠.			
1 11	_								
	- 1						. :		
12	· <u> </u>								
	_								
13	-	13.0' <b></b> -							· .
	_ [								
14	_		5	14' - 15.5'	8	10"	М	1.75	
	_	Grayish Green Silty CLAY		,					}
15	_	( Possibly Weathered From Granite )							
	_ ]	(USCS - CL, USDA - Clay Loam)							•
16	_			·					
	_	(Very Hard Drilling @ 17')							
17	_	17.0'		. *		1			
	_	Weathered Granite Bedrock							
18	_	(USCS - N/A, USDA - Weathered Rock)							
	-	(Auger Refusal @ 19.5')			1.2				
19	_	E.O.B. 19.5'	6	19' - 20.5'	50/1	1	W		
	_	Water at Completion 6.0' & Cave-in @ 8'							
20	_	Bore Hole filled with Bentonite Chips		.*	ŀ	<u>.</u>			
	lin T	esting Services, Inc.		:	<u> </u>		·	NTS#	141.04

# State of Wisconsin- Dept of Natural Resources P.O. Box 7921, Madison WI 53707-7921

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between \$10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

Route To:  Drinking Water Watershed \	Notor I	Wasta Man	agomont	Permediation/Rede	welonment	Other:					
Drinking Water Watershed \ 1. General Information	valet	vvaste iviaii	ayenem	Remediation/Redevelopment Other:							
The first profession and the second of the first of the f	D Na	Countr		Facility Name							
Boring Number DNR Well	D NO.	County Portage	<u>.</u>	UWSP Residence Hall							
Common Well Name	· · ·		# (if applic.)	Facility ID		Permit No.	City, Village, or Town				
				140.04			Stevens Point				
1/4 / 1/4   1/4   Section		Township		Street Address of We							
				Illinois Avenue							
Grid Location		I Grid Origin	1	Present Well Owner		Original V	Vell Owner				
Feet N E		mated)	OR			<u> </u>	· .				
Latitude.	Longitude	Location .		Street Address or Ro	ute of Owner						
DEG MIN SEC	DEG	MIN	SEC	City		State	ZIP Code				
<u> </u>	V		W		. *						
Reason For Abandonment	WI Unique	Well No. of Re	eplacement Well	Valentin and a second of the second design of the s	e passancia de la compansión						
				4. Pump, Liner, So							
3. Well / Drillhole / Borehole Informa	A CONTRACTOR			Pump and piping re	emoved?	Yes					
Monitoring Well		construction	Date	Liner(s) removed?		☐ Yes					
Water Well	7/7/0		Danastia	Screen removed?		☐ Yes	Pro vers				
■ Borehole / Drillhole		Construction	-	Casing left in place		☐ Yes	- Marie				
F-16-1	avaliable,	please attac	an.	Casing cut off below surface? Yes No NA							
Construction Type:    Drilled   Driven (sal	rdnoint)	T Dua		Sealing material rise to surface?							
☑ Drilled ☐ Driven (sa ☐ Other (specify):		☐ Dug	:	1			□ No ☑ N/A				
				If yes, was hole		Yes	□ No ☑ NA				
Formation Type				If bentonite chips w	ere used, were	e they	∏ No ☑ N/A				
Unconsolidated Formation	☐ Bed	rock		hydrated with water from a known safe source? Yes No NA  Required Method of Placing Sealing Material  Conductor Pipe-Gravity  Screened and Poured  Other (explain):							
Total Well Depth From Groundsurface	· ·	Caeina Dis	ameter (in.)								
/ / / / / / / / / / / / / / / / / / /	(it.)	Casing Di	arricter (iii.)								
Lower Drillhole Diameter (in.)		Casing De	enth (ff \	(Bentonite Chips)							
Conor Diminote Diameter (it.)	. *	Journa De	apar (ic)								
		1 .		Sealing Materials  Neat Cement Grout  Clay Sand Slurry (11lb/gal wt.)							
Was Well Annular Space Grouted?	Yes	□ No □	Unknown	Sand Cement (co		• .	Sand Slurry				
				☐ Concrete ☐ Bentonite Chips							
If yes, to what depth (feet)?	Depth to v	vater (feet)		For Monitoring Wells	and Monitorin						
	4.3	1		Bentonite Chips Granular Bentoni	ita		Cement Grout Sand Slurry				
	4.3			l'		s Sealant or	Mix Ratio or				
5. Material Used to Fill Well / Drillhol	9		From (ft.)	To (ft.)	Volume (ci	The Committee of the Co	Mud Weight				
3/8" Bentonite Chips			Surface	4.3'							
			<u>.</u>								
6. Comments			ou nominativa (a.c.) Aposto nominativa		<b>多性情報</b> 第						
Borehole collapsed @ 4.3 feet	when co	mpleted.	1. 1	•	2 92						
7. Supervision of Work					DNR (	Jse Only					
Name of Person or Firm Doing Sealing	<b>Nork</b>	Date of Ab	andonment	Date Received		Noted By					
NTS, Inc.		07/07/08	3								
Street or Route		Telephone		Comments							
P.O. Box 127	Cinto	(715) 34 ZIP Code	11-/9/4	Signature of Person Doing Work Date Signed							
City Stevens Point	State WI	54481		Signature of Person Doing Work Date Signed							
OLEVEI IS FUII IL	10th										

#### State of Wisconsin- Dept of Natural Resources P.O. Box 7921, Madison WI 53707-7921

# Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between \$10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

☐ Drinking	Water [	Watershed W	later	Waste Man	agement	Remediation	on/Redevelo	pment	Other:		····	
1. General I	nformation		Name (2019) et (	The Hallet agent the		2. Facility	/ Owner Info	ormation	3,3 %			
Boring Numb	er	DNR Well II	No.	County		Facility Name	)					
2				Portage	<b>.</b>	UWSP Re	sidence H	łali				
Common We				<del></del>	# (if applic.)	Facility ID		License/Pem	nit No.	City, Villac	e, or Town	
				(					Stevens			
1/4 / 1/4	1/4	Section		Township	Range	140.04 Street Addres	s of Well	<u> </u>				
				E W	Illinois Ave					,		
	Grid Location	<u>'</u> 1	Loca	al Grid Origin	<del>'</del>	Present Well			Original V	Vell Owner		
Feet	ГΝ	Е	(esti		OR .							
	Гѕ	Γw		Location	OK .	Street Addres	s or Route o	of Owner	1			
Latitude:	, , , ,		Longitude	<del> </del>	<u> </u>	Olicel, addies	o i route c	a Ottiloi				
DEG	MIN	SEC	DEG	MIN	SEC	City			State	ZIP Code		
		N		1	.w	1			1.			
Reason For A	Abandonment		WI Unique	Well No. of R	eplacement Well							
1			<b>!</b>			4. Pump, Liner, Screen, Casing & Sealing Material						
3. Well / Drif	lhole / Boreh	ole informati	on			Pump and	piping remov	red?	Yes	No	₩ N/A	
[ Monitori	ing Moll		Original C	construction	Date	Liner(s) ren	Liner(s) removed?					
<b>M</b> onitori			7/7/0	В		Screen rem	noved?		Yes	No	V NA	
☐ Water V	Veli		If a Well (	Construction	Report is					∏ No	₩ NA	
■ Borehole	e / Drillhole		evailable.	please attac	ch.							
Construction	Typo:		],	,		, , , , , , , , , , , , , , , , , , ,						
Construction		Driven (san	daaint)	- P.		_				F 4:	□ N/A	
	• •			Dug			ttle after 24 h		Yes	No	<b>V</b> N∕A	
Other (specify):					lfyes, w	as hole retor	pped?	Yes	∏ No	₩ NA		
Formation Type					If bentonite	chips were u	used, were they	/	III No	₩A		
Uncons	olidated Form	ation	☐ Bed	rock		hydrated with w	ater from a kno	own safe source?	l res	1	#11.8 1 <b>47</b> 1	
								ng Sealing Mat				
Total Well De	pth From Gro	oundsurface (f	t.)	Casing Di	ameter (in.)	Conducto	or Pipe-Grav	rity	Conductor	r Pipe-Pum	oed	
		· ·				Screene	d and Poure		Other (exp			
Lower Drillhol	e Diameter (ir	n )		Casing De	enth (ft \	(Bentonite						
	ii) ioloituic (ii	•• <i>y</i>		John S.	, pur (10.)							
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Stevens Po	oint		WI ·	5 <del>44</del> 81						]		

# State of Wisconsin- Dept of Natural Resources P.O. Box 7921, Madison WI 53707-7921

# Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between \$10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

Route To:					<b></b>		~			
				Remediation/Redevelopment Other:						
1. General Information					2. Facility / Owner Information					
Boring Number DNR Well ID No.			County		Facility Name UWSP Residence Hall					
Common Well Name			Portage		Facility ID	License/Pem	nit No	City, Village, or Town		
Contained work realise			Gov't Lot # (if applic.)		140.04	LIOCHSON	140.	Stevens Point		
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# State of Wisconsin- Dept of Natural Resources P.O. Box 7921, Madison WI 53707-7921

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

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CENTRAL WISCONSIN AREA:
3217 Whiting Avenue
P.O. Box 127
Stevens Point, WI 54481 -0127
(715) 341-7974 • Fax (715) 341-8654

MADISON AREA: 5620 Woodland Drive

Waunakee, WI 53597 (608) 849-9120 • Fax (608) 849-9122

# SUBSURFACE SOIL INVESTIGATION

UWSP HYER HALL 201 RESERVE STREET STEVENS POINT WISCONSIN

NTS 142.80

C09008

# PREPARED FOR:

ENGBERG ANDERSON 1 NORTH PINCKNEY STREET P.O. BOX 100 MADISON, WI 53703-0100

ATTENTION: MR. JAMES BROWN

# **FIELD INVESTIGATION BY:**

NUMMELIN TESTING SERVICES, INC. 3217 WHITING AVENUE STEVENS POINT, WI

MARCH 2, 2009



#### SUBSURFACE SOIL INVESTIGATION

# UWSP HYER HALL 201 RESERVE STREET STEVENS POINT WISCONSIN

#### 1. INTRODUCTION

Nummelin Testing Services, Inc. (NTS) performed this investigation to provide design information for the proposed University of Wisconsin – Stevens Point (UWSP) Hyer Residence Hall to be built at 201 Reserve Street in the City of Stevens Point, Portage County, Wisconsin. The results and recommendations reported are based upon information obtained during a field investigation with borings, and the geotechnical analysis of that information. This investigation follows a preliminary investigation that was done by NTS at this site in July of 2008. The report for the preliminary investigation was titled "Subsurface Investigation, Proposed UWSP Residence Hall, Illinois Avenue, Stevens Point, WI – NTS 141.04," and dated July 16, 2008. This report should be considered in its entirety during design and construction; no one section of the report should be used without considering all other sections.

The conclusions and recommendations reported are based on our interpretation of available subsurface and project information. The report may not represent variations that occur between or away from boring locations.

Should the scope of this project be altered, or if subsurface variations become evident during construction, it may be necessary to modify our recommendations. See the attached Geotechnical Engineering Report Information sheet for general information on NTS's geotechnical reports.

#### 2. PROJECT DESCRIPTION

The proposed project is the construction of a multi-level, dormitory-type, load-bearing masonry building at 201 Reserve Street, just north of the Allen Center. The east building wing is anticipated to be 6 stories with a lower level floor elevation of 1088. The west building wing is anticipated to be 5 stories with lower level floor elevation of 1093.

The maximum interior wall bearing load is estimated at 30 kips, and the maximum exterior wall bearing load is estimated at 15 kips per linear foot. Shallow foundations are anticipated, although caissons are also under consideration.

It is expected that up to three feet of fill will need to be added in the area of the west wing to bring the site to final grade, and some site grading is anticipated in the area of the east wing.

At the time of the investigation, the surface of the site was covered by four tennis courts, three volleyball courts, and several clusters of trees.

#### 3. FIELD INVESTIGATION

Fifteen standard penetration soil borings (Borings SB1 to SB15) were performed from February 16 to February 18, 2009, at locations shown on the attached sketch. Engberg Anderson determined the proposed boring locations and depths.

NTS located the borings in the field. Borings SB1 and SB14 encountered auger refusal at the proposed locations on obstructions at depths of 3 and 1 feet, respectively, and were moved up to 9 feet from the proposed locations and drilled again. Borings SB4 and SB11 had to be moved up to 23 feet from the proposed locations because trees and utilities prevented access to those boring locations. All other borings were drilled at the proposed locations.

Boring SB1 was proposed to be drilled to a depth of 35 feet, but auger refusal occurred in Boring SB1 at 12 feet on February 16, 2009. As requested, Boring SB1 was advanced by diamond-bit coring using an NQ-type barrel to a depth of 17 feet on February 25, 2009. Borings SB2 and SB3 were proposed to be drilled to a depth of 35 feet but were terminated when auger refusal occurred at depths of 37 and 43 feet, respectively, to provide a better profile of the rock should caissons be used for support. Borings SB4 through SB9, SB12, and SB14 were terminated at the proposed depth of 20 feet. Borings SB10, SB11, SB13, and SB15 were proposed to be drilled to a depth 20 feet, but were terminated at a depth of 25 feet to provide a better profile of the rock.

Representative soil samples were obtained while boring using the Standard Penetration Test (SPT) method according to ASTM Test Procedure D1586, at the depths indicated on the boring logs. Drilling between samples was by the hollow-stem-auger technique. The soils were visually/manually classified in general accordance with ASTM Test Procedure

D2488 by a technician at the time the borings were performed. The soil samples have been examined in the laboratory by this writer and classified according to the USCS and, as requested, the USDA soil classification system. Both the USCS and USDA classifications appear on the logs. Several samples were tested in the lab according to ASTM Test Procedure D4972 to determine pH. Other soil parameters and classifications reported herein have been estimated based on the standard penetration test results, soil descriptions, and experience with soils in this area. No other lab testing has been performed.

After completion of the borings, the bore holes were left open to obtain a 24-hour water level, and a temporary well was installed in Boring SB9. After the water level reading, all bore holes, except for the Boring SB9 bore hole, were backfilled with bentonite chips to comply with WDNR requirements, and the last few inches were patched with auger cuttings. The Boring SB9 bore hole will be backfilled with bentonite chips after the temporary well is removed.

At the time of the investigation, NTS determined ground elevations at the boring locations. The top of the large spout of the fire hydrant at the southeast corner of the site was used as a benchmark. According to the City of Stevens Point Engineering Department, the elevation of this benchmark is 1091.84.

Copies of the soil boring logs and a location sketch are appended to this report.

### 4. SUBSURFACE CONDITIONS

#### 4. 1. Area Geology

The subsoils in this area are mapped as outwash deposits. Outwash deposits generally consist of stratified sand and/or stratified sand and gravel. The underlying rock is mapped as granitic intrusive rock that is present at depths of less than 50 feet below the average surface terrain. A soil survey of Portage County maps the near-surface soils in this area as Roscommon muck that has been cut and filled at various locations. According to old soil maps, it appears that there used to be a creek near this area.

Note that mapped soil and bedrock conditions are provided for additional information only. Use of a design based on mapped or assumed conditions is not recommended.

# 4. 2. Soils at the Boring Locations

The general soil profile encountered by Borings SB1 to SB15 at this site was 6 to 10 inches of silty sand with organics (topsoil) over sand fill and/or sedimentary peat to

depths of 3 to 5.5 feet (elevation 1084.19 to 1088.84) over very loose to medium-dense, native sand to depths of 7.5 to 23 feet. Below the sand, stiff to hard clay and medium-dense clayey sand (probably weathered from the underlying granite) were encountered to depths of 16 to 34.5 feet. Weathered granite bedrock ('W Bedrock' in Table 1), was encountered in Borings SB1 through SB4, SB6 through SB9, and SB13 at depths of 7.5 to 34.5 feet (elevation 1055.04 to 1083.79). The degree of weathering of the bedrock varied from place to place and with depth, but generally decreased with depth. Auger refusal occurred in Borings SB1 through SB3 at depths of 12 to 43 feet (elevation 1046.54 to 1079.29). A summary of the soil profile encountered in each boring is presented in Table 1.

Table 1. Soil Profiles in the Borings.

Boring	Surf Elev	Fill	Peat	Native Sand	Clay/Sand	W Bedrock
SB1	1091.29	0'-3'		3' – 7.5'	<u> </u>	7.5' – 12'
SB2	1089.74	0' - 5.5'	2.5' - 3'	3'-18'	18' – 26'	26' – 37'
SB3	1089.54	0'-3'	6"-3	3' – 19'	19' – 34.5'	34.5' – 43'
SB4	1091.84	0'-2.5'	2.5' - 3'	3'-8'	8'-16'	16' - 20'+
SB5	1091.24	0'-2.5'	2.5' - 3'	3' – 7.5'	7.5' - 20'+	_
SB6	1090.64	0'-2.5'	2.5' - 3'	3'-8'	8'-13'	13'-20'+
SB7	1090.84	0'-2.5'	2.5' - 3'	3'-12'	12' - 16.5'	16.5' – 20'+
SB8	1090.44	0'-2.5'	2.5' - 3'	3'-13'	13' – 18'	18' - 20'+
SB9	1089.64	0'-2.5'	2.5' - 3'	3'-13'	13' – 18'	18' - 20'+
SB10	1091.54	0'-4.5'	4.5' – 4.7'	4.7' – 13'	13'-25'+	
SB11	1090.44	0'-5'	, <del>, , , , , , , , , , , , , , , , , , </del>	5' – 16'	16' - 25'+	_
SB12	1089.39	0' - 0.7'	0.7' – 3'	3'-17'	17' – 20'+	
SB13	1090.24	0'-3'	******	3'-20.3'		20.3' - 25'+
SB14	1089.34	0'-0.9'	0.9' - 3'	3'-18'	18' - 20'+	_
SB15	1088.19	0'-4'	2.5' - 2.7'	4'-23'	23' - 25'+	

Note that Borings SB1 and SB14 encountered obstructions in the fill at depths of 3 and 1 feet, respectively, and had to be redrilled at a nearby location. These obstructions appeared to be small slabs of concrete in the fill at these boring locations.

See individual boring logs for more detailed soil descriptions.

## 4. 3. Rock Coring Results

As requested, after auger refusal occurred in Boring 1 at 12 feet, Boring 1 was advanced by diamond bit coring to a depth of 17 feet. Results of the coring are shown below.

Recovery Ratio = 
$$0.650$$

$$RQD = 0.450$$

The core was of granitic rock which appeared to have softer layers in between hard layers, and fractures ran vertically as well as horizontally through the core sample.

#### 4. 4. Lab Test Results

As requested, samples from 3 borings were tested in the lab to determine pH according to Method A of ASTM Test Procedure D4972 using distilled water. Test results are presented below.

Boring / Sample	<u>Depth</u>	<u>pH</u>
SB1 / S2	3.5'	7.0
SB2 / S3	6.0'	7.2
SB14 / S2	3.5'	6.3

#### 4. 5. Water Level Measurements

Water was encountered in all bore holes during drilling at depths of 4 to 7 feet (elevation 1083.39 to 1085.64). Where the bore hole had not collapsed above the water table prior to the 24 hour reading, water levels in the bore holes were within a few inches of the level at the time of drilling.

At the location of Boring SB9, where the temporary well was installed, the water level after 24 hours and after 72 hours had not changed.

These water level measurements should be considered as representative of site conditions at the time the measurements were made. Seasonal fluctuations in the groundwater table of up to two feet may occur.

# 5. DISCUSSION AND RECOMMENDATIONS

#### 5. 1. General

Plan to completely remove all existing pavement, any existing foundations, topsoil, and trees from the building footprint prior to further excavation and site grading.

The soils at this site, with no modifications, are expected to provide variable support for shallow foundations. Peat and fill occurred in Borings SB1 through SB15 to depths of 3 to 5.5 (elevation 1084.19 to 1088.84), and in the preliminary borings at depths of 2 to 7 feet. Expect to do some undercutting to remove peat and/or fill from the base of excavations for shallow foundations, particularly those bearing at elevations of 1084 or higher. The amount of undercutting to remove peat and fill will depend on the proposed depth of the foundations. The native sand encountered below the fill and peat was very loose to medium-dense. Compaction of the native sand will probably be necessary at the base of excavations for shallow foundations.

Water was encountered at elevation 1083.84 to 1085.64 in Borings SB1 to SB15, and at elevation 1085.12 to 1085.67 in the preliminary borings at this site. However, it is expected that the water table may rise by up to 2 feet during periods of high groundwater, up to the proposed first floor elevation of 1088 of the east wing. To avoid water problems in the east wing, consider installing a drainage system below the east wing floor.

Expect dewatering to be necessary for excavations near or below elevation 1086 at this site.

See below for further recommendations.

# 5. 2. Foundations and Floor Slab Support

Standard spread footings may be used at this site, but be aware that undercutting and/or compacting of existing soils will probably be necessary for shallow foundations. Deep foundations, such as caissons or piling bearing on the granite bedrock may also be used. Note that the depth to harder granite increased from roughly 12 feet at the west end of the building (Boring 1) to 43 feet at the east end of the building (Boring 3). Intermediate foundations (rammed aggregate piers) may also be considered.

A review of construction plans and records, if available, for nearby buildings may be beneficial for determining the most economical foundation type.

It is recommended that all load-bearing soils be inspected by NTS or another qualified soils engineering firm to verify that adequate bearing is present.

If the recommendations in this report are followed, foundation total settlement should not exceed one inch, and foundation differential settlement should not exceed one-half inch.

#### 5. 2. 1. Shallow Foundations

It is recommended that shallow foundations bear on the native sand or on fill meeting the requirements of section 5. 4. that bears on the native sand. Note that the native sand was very loose in some borings, and undercutting/compaction of the native sand should be expected below shallow foundations. A presumptive allowable bearing capacity of 3,000 pounds per square foot (psf) may be used in the design of footings if the soil layer just below the footings is comprised of native sand and/or fill that meets these two requirements: the thickness of the layer must be at least one footing's width or 18 inches, whichever thickness is greater, and all soil in the layer must be compacted to at least 95% of the modified Proctor density (ASTM D-1557). Note that undercutting will likely require dewatering.

Shallow foundations should not bear on the peat or existing fill. Peat and/or fill were encountered in Borings SB1 through SB15 to depths of 3 to 5.5 (elevation 1084.19 to 1088.84), and in the preliminary borings at depths of 2 to 7 feet. Expect to undercut where peat and/or existing fill are found at the base of shallow foundation excavations.

# 5. 2. 2. Intermediate Foundations (Rammed Aggregate Piers)

NTS is not aware of rammed aggregate piers used for other buildings in Stevens Point, but sites with loose sands and high water table, such as this site, may be a good candidate for rammed aggregate piers. Rammed aggregate piers are constructed by ramming thin layers of aggregate into the ground with a high energy tamper. This forms a dense column of aggregate that offers support for bearing pressures of up to 10,000 psf. Rammed aggregate piers can also provide liquefaction mitigation. For more information regarding rammed aggregate piers, contact Steve Weyda of Geopier (262-628-1663).

#### 5. 2. 3. Caissons

Caissons bearing on the harder bedrock encountered at the end of Borings SB1 through SB3 may be used for support. The depth to harder granite increased from roughly 12 feet at the west end of the building (Boring 1) to 43 feet at the east end of the building (Boring 3). Note that the bedrock depth and degree of weathering have

also been found to vary significantly over short distances in other areas around Stevens Point, and may vary between borings.

Caissons should be drilled to refusal in the granite bedrock. Because of the sandy soils and shallow water table at this site, drilled shafts should be cased and no belling should be considered. If casing is omitted or belling is used, the shaft will likely collapse, and adjacent locations will be undermined as soil migrates horizontally into the hole. A slurry replacement method may also be necessary for drilled shafts if soil and water seeps into the casing after the casing has been socketed into the rock. Based on coring results and experience with rock in this area, the recommended end bearing is 8 tons per square foot for design of caissons drilled to refusal in the bedrock. The base of each caisson should be thoroughly cleaned and, where possible, verified to be free of loose rubble/soils. Any verification of the caisson must follow OSHA regulations. The caisson drill must be capable of removing rock pieces of a diameter up to one-third of the diameter of the caisson. For a higher bearing pressure recommendation at a caisson base, a small diameter caisson load test may be performed. Loading to at least twice the design pressure value is recommended during the test. Caissons bearing on similar rock have been used at other buildings at the UWSP campus, such as the UWSP Fine Arts building. Construction records for these buildings may help in selecting a higher bearing value.

# 5. 2. 4. Piling

Either driven or augered piling may be used for support.

Driving resistances for maximum structural capacity of piles such as 10x42 H piles and 10.75-inch cast-in-place closed-end-pipe piles are expected to occur near the depths of auger refusal in the borings, which varied from roughly 12 feet at the west end of the building (Boring 1) to 43 feet at the east end of the building (Boring 3).

Where vibration may cause movement of existing adjacent buildings, augered piles may be constructed with little or no vibration. Where no load testing is performed, the recommended end bearing for augered piles bearing on the harder granite is 10 tons per square foot. For a higher augered pile end bearing pressure recommendation, a load test may be performed. Loading to at least twice the design pressure value is recommended during the test.

# 5. 2. 5. Floor Slab Support

The soils at this site are expected to provide variable support for a floor slab. After excavation for the floor, proof-roll the soils in the floor area. Proof-rolling will increase the density of the soils, providing more uniform support, and help to identify

weak soils that are not suitable for support of the floor. An acceptable proof-roller for granular soil would be a smooth-drum, vibratory roller. Consider retaining NTS to observe the proof-rolling. Plan to undercut where weak soils are found. A subgrade modulus of 230 pounds per square inch per inch may be used for sand subgrades at this site which do not yield to the proof-roller.

Proof-rolling of soils near the water table at this site may cause the soils to become quick (liquefaction). The water table should be dropped at least 18 inches below the exposed grade prior to proof-rolling.

As an alternative to proof-rolling and undercutting, the floor slab may be structurally supported.

#### 5. 2. 6. Corrosion Potential

Results of pH lab tests on the sands at depths of 3 to 6 feet at this site ranged from 6.3 to 7.2, indicating the corrosion potential of steel in the sands to be low to moderate. However, peat and other organic soils were encountered above these depths. The corrosion potential of steel in the organic soils is likely to be high. Any construction materials that will be placed in contact with the organic soils should be protected against corrosion.

# 5. 3. Drainage System

During periods of high groundwater, the water table may rise up to the proposed first floor elevation of the east wing. To avoid water problems in the east wing, consider installing a drainage system and a vapor barrier below the east wing floor.

Drain tile for the system should be spaced at a frequency of no less than one drain per 15 feet, and all drain tiles should be part of one system. Several inches of sand conforming to ASTM C33 should be used as a filter around the drain tile and as the layer just beneath the floor. Clear stone or pea gravel does not work as a filter, and, although some codes require gravel for drains, neither stone nor gravel should be used for drains at this site.

Do not connect the outlet of a gravity-flow drainage system to any other storm drain systems because water may flow backward through the drains and into the building during periods of heavy stormwater flow. Consider directing the flow to an exterior manhole where the water could be pumped out with a portable pump during periods when the water table is high.

## 5. 4. Compaction and Fill Requirements

The existing sand (both fill and native) encountered, if unsaturated and free of deleterious materials, is acceptable for use as structural fill. The peat encountered should not be used as structural fill. If imported fill is required, we recommend the fill be clean, unsaturated, granular soil. At the time of construction, NTS or another qualified soils engineering firm should verify that the proposed fill soils are acceptable. This firm should verify that the moisture content is appropriate for proper compaction and that the fill contains no deleterious materials. Frozen soil should not be used as structural fill.

Any required fill should be placed in lifts not exceeding 1 foot (uncompacted).

Compaction of loose sand near or below the water table may cause the sand to become quick (liquefaction). If compaction of the sand near the water table is necessary, drop the water table at least 18 inches below the proposed grade and use light compaction equipment. To achieve required compaction levels with light equipment, lifts thinner than 1 foot may be necessary.

Compact all fill placed to at least 95% of the maximum dry density (modified Proctor method - ASTM D-1557). Site or soil conditions at the time of construction may warrant a change in the recommended compaction levels. However, no changes should be made without review by NTS or another qualified soils engineering firm.

#### 5. 5. Excavation

All excavations should comply with OSHA regulations. Excavation sidewalls in the onsite sands should have a slope no steeper than 1.5 to 1.0. To minimize sidewall cave-in during construction, particularly during vibratory compaction, reduce the sidewall slope to approximately 30 degrees from the horizontal. Where steeper sidewall slopes are required or more convenient, temporary shoring will probably be necessary. Temporary shoring is generally left up to the contractor to design and construct.

Most common excavators (backhoes) should be adequate to make the necessary excavations.

Undercutting for non-structurally-supported floors and shallow foundations should be expected at this site. When undercutting, use the sixty-degree approximation to determine the resulting pressure at the base of the undercut. The recommended width of undercut is twice the undercut depth plus the width of the footing or floor. If the footing/floor location is accurately marked and centered in the base of the undercut, then the minimum width of the undercut is the depth of undercut below the footing plus the width of

structure, measured at the base of the undercut. A good practice is to add at least one foot to this width. Replace all undercut soils with properly compacted fill (see section 5. 4. 'Compaction and Fill Requirements'). Compact any disturbed soil at the base of excavations/undercuts with a high energy compactor such as a hoe-pack or vibratory roller.

Excavations should be performed, where possible, using a flat plate attached to the bucket teeth of the backhoe to minimize disturbance at the base of the excavations. Where a toothed bucket is used, the last six inches or so should be excavated by turning the bucket so that the teeth are parallel to the proposed grade. Compact the base of all excavations.

Expect to encounter groundwater in excavations near and below elevation 1086. Dewatering will probably be required where water is encountered.

#### 5. 5. 1. Excavation Dewatering

Dewatering will probably be necessary for excavations that extend below the water table. Where excavations are anticipated to extend below the water table, dewatering prior to excavation is recommended. When dewatering, avoid pumping water from within the excavation. Pumping from within the excavation may loosen the soil at the base of the excavation as surrounding water flows into the excavation. This loosening may not be obvious during construction and can cause significant settlement of foundations. To avoid loosening soil at the base of the excavation, dewater with deep wells or well points from outside the excavation.

It is recommended that the water table be dropped at least 18 inches below the base of the excavation. Should the base of any excavation be at or slightly below the water table, roughly twelve inches of well graded, breaker-run rock (less than 10 percent P200 material) may be placed at the base of the excavation to provide a stable working platform and help prevent loosening of soils at the base of the excavation. The breaker-run should be placed and compacted immediately following excavation, and excavation before breaker-run placement should be limited to small areas.

### 5. 5. 2. Existing Structures

Care must be taken when excavating next to existing structures. Do not excavate soil under a line drawn out (away from existing structure) and down from the top of a footing at a 45-degree angle, unless proper precautions are taken. If excavations will extend below the elevation of the existing footings, the existing soil under the structure may have to be shored. This may be done using sheet piling, properly braced or tied back. Allow for imposed lateral loads from nearby footings in designing the system. Be aware that vibrations during driving of sheet piles may cause loose soils

below the existing structure to settle. Monitor the existing structure for possible movement during the construction process. Construction records for the existing structure should be reviewed, if available. These may show engineered compacted granular fill below and outside existing footings. If this is the case, risk and severity of problems may be reduced if excavation below the elevation of the existing footing is required. Excavating small sections at a time may reduce the potential for seriously undermining the existing footing, and may allow construction to continue without the need to brace or use sheet piles.

Caution should be used when compacting adjacent to existing footings. Vibratory compacting equipment may cause some settling in adjacent soils below existing foundations. It may be better to use light compaction equipment near existing footings.

## 5. 6. Lateral Earth Pressures

Unsaturated, firm, clean sand (unsaturated on-site sands and imported fill) will exert approximately 35 pounds per cubic foot (pcf) equivalent fluid pressure in the active state, approximately 330 pcf equivalent fluid pressure in the passive state, and approximately 55 pcf equivalent fluid pressure at rest.

Be aware that a saturated soil may exert up to three times as much pressure as a non-saturated soil, and successive passes of a compactor near earth-retaining structures may build up pressures significantly higher than those presented above.

# 5. 7. Coefficient of Static Friction for Sliding Analysis

For cast-in-place concrete against the sands at this site, a coefficient of static friction of not more than 0.577 should be used in conjunction with a safety factor against sliding of 1.5.

For pre-cast concrete against the sands at this site, a coefficient of static friction of not more than 0.462 should be used in conjunction with a safety factor against sliding of 1.5.

# 5. 8. Site Classification for Seismic Design

Most sand at this site is loose and below the water table, making the sand potentially liquefiable. According to Table 1615.1.1 of the 2002 Wisconsin Enrolled Commercial Building Code, the site classification for this site would be Site Class 'F' because of these potentially liquefiable soils.

# 5. 9. Pavement Design Considerations

The following section may be used in the design of any pavement at this site.

It is anticipated that the subgrade soils for the parking lot will consist of either the on-site sand fill or imported clean, fine sand fill. The on-site sand fill was very loose in some areas. It is recommended that the pavement subgrade be thoroughly compacted prior to base course placement.

The sand is slightly frost susceptible and has a Frost Group Designation of F-2. A CBR test was not performed. However, the CBR factor for this material based on the soil description is estimated to be 15.0. The Wisconsin Asphalt Pavement Association Design Guide suggests a soil support value of 5.0 for soils with a CBR value between 10 and 20, classifying the subgrade as 'good'. A subgrade modulus of not more than 230 pounds per square inch per inch should be used for pavement and slab design on these soils.

If asphaltic pavement is used, design thicknesses will vary with expected traffic loads. The traffic load refers to the expected design daily ESALs (18,000 pound Equivalent Single Axle Loads). The following suggested pavement and crushed aggregate base thicknesses are based on 1 to 5 design daily ESALs (usually residential streets, car parking lots, and similar traffic loads).

#### 5. 9. 1. Flexible Pavement, 1 to 5 Design Daily ESALs

Use a minimum of 8 inches of crushed aggregate base course with a minimum of 3 inches of asphaltic concrete.

If the expected daily ESALs are greater, plan to increase these thicknesses.

Rigid (Portland cement concrete - PCC) pavement may be used. Below normal parking areas, a slab thickness of at least 6 inches is recommended. PCC pavement should also be used at locations of trash dumpsters or other heavy-duty traffic locations. A thickness of at least 8 inches is recommended for concrete slabs in heavy-duty traffic areas. The base course thickness below the slab is not critical, but it should be thick enough to provide stability for the slab.

The pavement construction should meet the requirements of the Wisconsin DOT Standard Specifications for Road and Bridge Construction.

A prime requirement for successful pavement is preparation of the subgrade soil. At the time the base course is being placed, the subgrade should be firm when proof-rolled. Proof-rolling of the proposed pavement subgrade will help identify areas containing peat or other weak soils that may require undercutting. An acceptable proof-roller for sand would be a large, smooth-drum, vibratory compactor. The subgrade may yield slightly to

the proof-roller, but after base course placement the base grade should be unyielding to the fully loaded, tandem-axle, dump trucks. This requirement also applies after the completion of any undercut.

It may be necessary to undercut and replace soft or loose soils with crushed rock or breaker-run rock. Any rock used to stabilize a soft subgrade should not be considered as part of the base course thickness.

Respectfully,

bkn/cerl/bn/mn

Benjamin K. Nummelin, P.E.

Beyon H. Monnolis

Nummelin Testing Services, Inc.

Clifton E.R. Lawson, P.E.

Cliffm ER Lawson /mn

**Consulting Engineer** 

# NUMMELIN TESTING SERVICES, INC

#### GEOTECHNICAL ENGINEERING REPORT INFORMATION SHEET

Subsurface soil conditions are responsible for many of the construction problems encountered at building sites. In order to help you, our client, manage your risks, we offer you the following information and suggestions.

Geotechnical engineering reports are based on observations of specific soil conditions existing at the time of the subsurface soil investigation. As these conditions may change over time, construction decisions should be made with the timeliness of the report in mind. Further testing may be advisable if subsurface soil conditions are affected by natural events (flooding, spring thaws, etc.) and construction (drilling, blasting, surcharges, etc.) on-site or adjacent to it. Talking to your geotechnical professional before construction begins will help keep one informed if further tests are recommended.

The recommendations included in your geotechnical engineering report are based on a limited number of samples/tests. These recommendations assume that subsurface conditions throughout the site will be similar to those observed. As all recommendations are preliminary when based on limited testing, it is important to have your geotechnical professional observe the actual conditions during construction. This allows him/her to note any differences that may not have been revealed by the limited samples/tests and/or that are more abrupt than reported in the preliminary report. It is this geotechnical professional, using his/her knowledge and familiarity of site history, as well as construction observations, who will be able to determine if there is adequate and appropriate support to consider these recommendations final. He/she will also be able to document that the contractor is following these recommendations. Be aware that this geotechnical professional can not assume responsibility and/or liability for his/her recommendations based on observations and determinations by others.

Professional judgement, based on experience and observations, is at the heart of our geotechnical recommendations. Geotechnical reports use information from a limited number of samples/tests to predict conditions regarding your overall site. No one may say with certainty what subsurface conditions really exist without actual observation. The conditions away from sample/test areas may vary from what is predicted. It is important to identify variations as early as possible. This is why we encourage you to take advantage of our knowledge and experience during the construction phase of your project. Working together we can help minimize the impact when unexpected variations occur.

Geotechnical reports are written for a specific client, purpose, project and set of conditions. They are not intended to be a generalized, generic report for a proposed site. They are for the sole use of our client for the express purpose indicated to us. Should the scope of the project be altered, or if subsurface variations become evident during construction, it may be necessary to modify our recommendations. Early communication with your geotechnical professional can help you avoid expensive problems that may occur when changes to a project's purpose, structure, size, usage, site orientation, elevation, etc. are made after a report is written.

Following these guidelines, your geotechnical subsurface report should provide informed and accurate information to assist in the planning and construction of your project.

Soil Boring LOCATIONS

#### **BORING LOG NOTES**

#### DESCRIPTIVE TERM, GRANULAR SOIL (% BY DRY WEIGHT)

Trace 0% - 5% Little 5% - 12% Some 12% - 35% And 35% - 50%

Q<sub>P</sub> = Estimated Unconfined Compressive Strength (by pocket penetrometer) expressed in tons per square foot (t/sf).

Q<sub>U</sub> = Estimated Unconfined Compressive Strength (by ASTM 2166) expressed in tons per square foot (t/sf).

NM = Natural Moisture

M = MOISTURE

D = Dry F = Frozen
M = Moist W = Wet
S = Saturated

LOI = Loss on Ignition (Organic Content)

N (Standard Blow Count) = blows per foot, as shown. Performed in general accordance with Standard Penetration Test Specifications (ASTM D-1586).

NR = No Recovery WOH = Weight of Hammer # = Sample Number

**SOIL CLASSIFICATION** 

F = Fine LL = Liquid Limit, percent

M = Medium PL = Plastic Limit, percent

C = Coarse PI = Plasticity Index (LL-PL)

W.L. = Water Level

#### SOIL STRENGTH CHARACTERISTICS

CONSISTENCY	(Cohesive Soils)	RELATIVE DENS	SITY (Granular Soils)
<u>Term</u>	$Q_u$ tons/sq.ft.	<u>Term</u>	"N" Value
Very Soft	0.0 to 0.25	Very Loose	0 - 4
Soft	0.25 to 0.50	Loose	4 - 10
Firm	0.50 to 1.0	Medium-Dense	10 - 30
Stiff	1.0 to 2.0	Dense	30 - 50
Very Stiff	2.0 to 4.0	Very Dense	Over 50
Hard	Over 4.0	•	

#### ORGANIC CONTENT BY COMBUSTION METHOD PLASTICITY

Soil Description	Loss On Ignition	<u>Term</u>	Plastic Index
Non Organic	Less than 4%	None to Slight	0 - 4
Organic Silt / Clay	4 - 12%	Slight	5 - 7
Sedimentary Peat	12 - 50%	Medium	8 - 22
Fibrous & Woody Peat	More than 50%	High to Very High	Over 22

Boring By: Nummelin Testing Services, Inc.

UWSP Hyer Hall Moved 9'N - See Plan 201 Reserve St, Stevens Point, WI Project: Location:

Boring: SB1 Auger: **HSA** Page: 1 of 1 Drillers:

NH/MA Date: 2/16/2009 Elevation: 1091.29

-		201 Reserve St, Stevens Point, WI						vation:	1091.29
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	М	Qp (tsf)	Notes
<u> </u>	-	Dark Brown Silty SAND w/ Organics		1	<del>                                     </del>	\/		\- <del>/</del>	
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	18	18	F	<b>]</b> i	
_	_	8"	-		-	- *			
2	_	Dark Brown Fine SAND, Some Silt							
	_	(Poss Fill) (USCS: SM) (USDA: Loamy Sand)							
3	_	(Obstruction@3' - Moved 9'N & Drilled Again)			1				
	_	3.0'	2	3.5 - 5	12	18	М		
4		3,0	-	5.5 - 5	12	10	171		
"									
5		Light Brown Mostly Fine SAND							
'	_	( USCS: SP ) ( USDA: Sand )			1				
6	-	( OSCS: SF ) ( OSDA: Saild )	2	675	7	10	c		
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		7.5' <b></b> -							i
8	-	CONTAINING A STATE							
	-	GRAVEL w/ Cobbles	,	0 10 5			_		
9	-	(USCS: GP) (USDA: Gravelly Sand)	4	9 - 10.5	45	3	S		
	-	(Probable Weathered Granite Bedrock)							
10	-	( Hard Drilling @ 10')							
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	-	( Auger Refusal @ 12')		,					
12		( Boring Advanced By Diamond Bit Coring )							į
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17	<b>~</b>	E.O.B. 17.0'	j					ſ	[
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18	-	Water @ EOB 5.8', 24hr 5.7'	l			l		I	
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Boring By: Nummelin Testing Services, Inc.

Project:

UWSP Hyer Hall

Location: See Plan

201 Reserve St. Stevens Point, WI

Boring: Auger: Page:

SB2 HSA 1 of 2 NH/MA

**Drillers:** NH / MA **Date:** 2/16/2009 **Elevation:** 1089.74

		201 Reserve St, Stevens Point, WI						ation:	1089.74
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
1	-	Dark Brown Silty SAND w/ Organics (USCS: SM) (USDA: Loamy Sand)	Ī	1 - 2.5	7	18	М		
2	-								
3	-    -    -	(Black Peat Layer 2.5' - 3.0') Dark Brown Fine SAND, Little Silt (Poss Fill) (USCS: SP-SM) (USDA: Sand)	2	3.5 - 5	2	18	W		
4	-	(Toss Pill) (Obco. br-bivi) (ObdA, balld)	<i>L</i>	3.3 - 3	2	18	VV		
5	-	5.5'							
6	-		3	6 - 7.5	5	18	S		
7	- -								
8	- -								
9	-		4	9 - 10.5	7	18	S		
10							~		
11	-	Light Brown Mostly Fine SAND							
12	-	(USCS: SP) (USDA: Sand)							
13	-								
	1		ا ـ	14 17 7	10	10			
14	-		5	14 - 15.5	13	18	S		
15	-								
16	-								
17	-								
18	-	18.0 <sup>+</sup>						İ	
19	-		6	19 - 20.5	13	18	S	1.5	
20	-								
21	-	Brownish Red CLAY Little Gravel							
22	-	( USCS: CH ) ( USDA: Clay ) ( Probably Weathered From Granite )							
23	-	( Flobably weathered Flom Granne)						İ	
24	-		7	24 - 25.5	10	18	S	4.5	
25	-	( Continued )							
Numme	lin T	esting Services, Inc.						NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

Project:

Location:

UWSP Hyer Hall See Plan 201 Reserve St. Stevens Point, WI

Boring: Auger:

2 **HSA** 2 of 2

Page: Drillers: Date:

NH/MA 2/16/2009

		201 Reserve St, Stevens Point, WI						vation:	1089.74
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)		Qp (tsf)	Notes
	-	( Continued )	<b>-</b>		<b>_</b>				
26	-	26.0'							
0.77	-	( Hard Drilling @ 26')							
27	-		1						
28	] _			]					
20									
29	_		8	29 - 30.5	34	18	S	4.5	
	_			2/ 30.3	´ '	10	Ü	7.5	
30	-	Brownish Red CLAY							
	-	Little Gravel							
31	-	(USCS: CH)(USDA: Clay)							
32		( Probable Weathered Granite Bedrock )	Ì	!					
32									
33	_								
	-								
34	-		9	34 - 35.5	50/3	9	S		
	-					-			
35	-								
36	-						i		
30	-	( Auger Refusal @ 37')							
37	_	E.O.B. 37.0'							
	-	Borehole Filled with Bentonite Chips							l
38	-	Water @ EOB 5.0', 24hr 4.9'					ļ		]
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Boring By: Nummelin Testing Services, Inc.

Project:

UWSP Hyer Hall See Plan

Location: See Pla

201 Reserve St, Stevens Point, WI

Boring: Auger:

Page:

SB3 HSA 1 of 2 NH/MA

Drillers: Date: Elevation:

2/17/2009 1089.54

		201 Reserve St, Stevens Point, WI						vation:	1089.54
Depth	]	Classification/Description	#	Sample	N	Rec	M	Qp	Notes
(ft)				Depth(ft)		(in)		(tsf)	
	-	Brown F - M SAND (Fill)						1	
1	-	(USCS: SP)(USDA: Sand)	1	1 - 2.5	6	18	M		
1	-	6 <sup>11</sup>				İ			
2	-	Black Organic SILT						ŀ	
	-	(Poss Fill) (USCS: Pt) (USDA: Muck)		Į.					
3	-	3.0'	1						
	-		2	3.5 - 5	7	18	M		
4	-		i						
_	-	Light Brown F - M SAND			l				
5	-	(USCS: SP)(USDA: Sand)			]				
	-		1						
6	-		3	6 - 7.5	4	18	S		
	-								
7	-	7.0 <b>'</b> -							
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41	_	Brown/Orange Clayey SAND Some Gravel			ĺ			- 1	ĺ
22	-	( USCS: SC ) ( USDA: Sandy Clay Loam )						]	ļ
44	-	( OSCS: SC ) ( OSDA: Sandy Clay Loam ) ( Probably Weathered From Granite )		I		]			
23	- 1	( riouadly weathered from Granne)		ĺ		l		1	İ
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	din T	esting Services, Inc.			1			KITC II	177 01
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Boring By: Nummelin Testing Services, Inc.

Project:

Location:

UWSP Hyer Hall See Plan 201 Reserve St. Stevens Point. WI

Boring: Auger:

SB3 HSA 2 of 2

Drillers: Date:

Page:

NH/MA 2/17/2009

- <del> </del>		201 Reserve St, Stevens Point, WI						vation:	1089.54
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)		Qp (tsf)	Notes
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29	-	Brown/Orange Clayey SAND Some Gravel		20 20 5			_		
2.9	] [	(USCS: SC) (USDA: Sandy Clay Loam)	8	29 - 30.5	20	18	S		
30	_	(Probably Weathered From Granite)				ļ			
"	_	(Trobably Weathered From Granite)		ĺ	ļ		İ		
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37	-								
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38		Brown/Orange Clayey SAND Some Gravel							
] 30	_	( USCS: SC ) ( USDA: Sandy Clay Loam )							
39	_	(Probable Weathered Granite Bedrock)	10	39 - 40.5	50/2	8	s		
	-	( Tradition of the Bouroux)	10	37 - 40.3	30/2	°	ာ		
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	-	(Auger Refusal @ 43')					- 1	İ	
43	- [	E.O.B. 43.0 <sup>t</sup>				İ	į	-	
1 11	-	Borehole Filled with Bentonite Chips							
44	-	Water @ BOB 5.0', 24hr 5.1'				-			
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numme	un 1	esting Services, Inc.					1	VTS#	142.80

Boring By: Nummelin Testing Services, Inc.

Project:

UWSP Hyer Hall

Moved 21'N and 9'E - See Plan 201 Reserve St. Stevens Point, WI Location:

Boring: Auger: Page: Drillers:

**HSA** 1 of 1 NH/MA 2/16/2009

SB4

Date: Elevation.

Columbia   Columbia			201 Reserve St, Stevens Point, WI						vation:	1091.84
1	Depth (ft)		Classification/Description	#		N			Qp (tef)	Notes
1	(20)		Dark Brown Silty SAND w/ Organics		Depth(1t)		(111)		(131)	
2	1 1			1	1-25	7	18	М		
Career   C		-	8"	1 1	1 2.3	, ,	'	171		
Carayish Brown CLAY   Some Gravel   Cuscs: CL) (USDA: Clay)   Cuschered From Granite)   Cuscs: CL) (USDA: Clay)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cuscs: CL)   Cuschered Gravel   Cusc	2	-	_							
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Brown Mostly Fine SAND (USCS: SP) (USDA: Sand)  6 -	3	-					l			
Brown Mostly Fine SAND (USCS: SP) (USDA: Sand)  6 -  7 -  8 -  8 -		-		2	3.5 - 5	18	3	М	} }	
Comparison of Comparison of	4	_					-			
Comparison of Comparison of		-	Brown Mostly Fine SAND							
7 - 8 8.0' 4 9 - 10.5 18 18 S 3.0  10 - 11 - Grayish Brown CLAY Some Gravel (USCS: CL) (USDA: Clay) (Weathered From Granite)  13 - (Hard Drilling @ 16') 16.0' 16.0' 17  15 - Grayish Brown CLAY Some Gravel (USCS: CL) (USDA: Clay) (Probable Weathered Granite Bedrock)  19 - (Probable Weathered Granite Bedrock) - Borehole Filled with Bentonite Chips Water @ 6.2', 24hr 6.2' 23  24 - 25 - 8.0' 8.0' 18 S 1.5	5	-	(USCS: SP)(USDA: Sand)							
7 - 8 8.0' 4 9 - 10.5 18 18 S 3.0  10 - 11 - Grayish Brown CLAY Some Gravel (USCS: CL) (USDA: Clay) (Weathered From Granite)  13 - (Hard Drilling @ 16') 16.0' 16.0' 17  15 - Grayish Brown CLAY Some Gravel (USCS: CL) (USDA: Clay) (Probable Weathered Granite Bedrock)  19 - (Probable Weathered Granite Bedrock) - Borehole Filled with Bentonite Chips Water @ 6.2', 24hr 6.2' 23  24 - 25 - 8.0' 8.0' 18 S 1.5		-					İ			
8 8.0' 4 9 - 10.5 18 18 S 3.0  10 - 11 - Grayish Brown CLAY Some Gravel (USCS: CL) (USDA: Clay) (Weathered From Granite)  12 - (USCS: CL) (USDA: Clay) (Weathered From Granite)  14 - 5	6	-		3	6 - 7.5	13	18	S		
8 8.0' 4 9 - 10.5 18 18 S 3.0  10 - 11 - Grayish Brown CLAY Some Gravel (USCS: CL) (USDA: Clay) (Weathered From Granite)  12 - (USCS: CL) (USDA: Clay) (Weathered From Granite)  14 - 5	_	-								
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Some Gravel (USCS: CL.) (USDA: Clay) (Weathered From Granite)	10	_								
Some Gravel (USCS: CL.) (USDA: Clay) (Weathered From Granite)	11	_	Gravich Proven CLAV							
12 - (USCS: CL) (USDA: Clay) (Weathered From Granite)  13 - (Weathered From Granite)  14 - (Some Gravel) (USCS: CL) (USDA: Clay) (Probable Weathered Granite Bedrock)  20 - E.O.B. 20.0' Borehole Filled with Bentonite Chips 21 - Water @ 6.2', 24hr 6.2'  23 - (USCS: CL) (USDA: Clay) (Probable Weathered Granite Bedrock)  24 - (Some Gravel) (Probable Weathered Granite Bedrock)  25 - (Some Gravel) (Probable Weathered Granite Bedrock)  26 - (USCS: CL) (USDA: Clay) (Probable Weathered Granite Bedrock)  27 - (Some Gravel) (Probable Weathered Granite Bedrock)  28 - (Some Gravel) (Probable Weathered Granite Bedrock)  29 - (Some Gravel) (Probable Weathered Granite Bedrock)  20 - (Some Gravel) (Probable Weathered Granite Bedrock)  21 - (Some Gravel) (Probable Weathered Granite Bedrock)  22 - (Some Gravel) (Probable Weathered Granite Bedrock)  23 - (Some Gravel) (Probable Weathered Granite Bedrock)  24 - (Some Gravel) (Probable Weathered Granite Bedrock)	11									
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Company   Comp		-					- ~	~		
16 16.0'	15	-								
17 - 18 - 18 - 19 - 19 - 19 - 10 (Probable Weathered Granite Bedrock)  20 - 21 - 22 - 23 - 24 - 25 -		-	( Hard Drilling @ 16')					1		
18	16	-	16.0'						I	
18	<b>.</b>	-			1					
18	17	-								
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19 - (Probable Weathered Granite Bedrock) 20	18	-			10.5.50			ا پر	Ì	
20 E.O.B. 20.0' Borehole Filled with Bentonite Chips Water @ 6.2', 24hr 6.2'  22	10	-		6	18.5 - 20	32	18	$ \mathbf{S} $		
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21 Borehole Filled with Bentonite Chips 22	20	_	E O D 20 M		ĺ					
21 Water @ 6.2', 24hr 6.2'  22	20	_						- 1		
22 - 23 - 24 - 25 -	21	_			l	l	l		1	İ
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Nummelin Testing Services, Inc. NTS # 142.80		<u>-  </u>								
	Numme	tin T	esting Services, Inc.						NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

Project:

UWSP Hyer Hall See Plan

Location:

201 Reserve St, Stevens Point, WI

Boring: Auger: Page:

SB5 **HSA** 1 of 1

Drillers: Date: Elevation:

NH/MA 2/16/2009 1091.24

Depth	T	Classification/Description	T #	Sample	ŢΝ	Rec		Qp	Notes
(ft)			"	Depth(ft)	"	(in)	1	(tsf)	Notes
<del></del>	-	Dark Brown Silty SAND w/ Organics	<del> </del>		<del> </del>	,	<del> </del>	(101)	
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	4	18	M		
	-	10"	1	1	'		```		
2	-	Dark Brown Fine SAND, Little Silt						1	
	1 -	(Fill) (USCS: SP-SM) (USDA: Loamy Sand)							
3		2.5'							
} ~	_	(Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	6	18	М		
4		(Diack real Layer 2.3 - 5.0)	2	3.5 - 5	U	10	171		
1 7									
5	-	T T. MAGANIEN	1				ļ		
)	-	Tan F - M SAND			1				
	-	(USCS: SP)(USDA: Sand)							
6	-		3	6 - 7.5	8	18	S		
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9	-		4	9 - 10.5	7	18	S		1
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10	_	Grayish Brown F - M Clayey SAND							
		(USCS: SC) (USDA: Sandy Clay Loam)			ŀ				
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16	-	Brownish Red F - M Silty SAND	ı						
	-	Some Gravel					Ì		1
17	-	( USCS: SM ) ( USDA: Loamy Sand )		:					
1	-	( Probably Weathered From Granite )							
18	-		ł	ĺ			İ	I	
1	-		6	18.5 - 20	20	18	S	1	
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20	-	E.O.B. 20.0'					l		
	-	Borehole Filled with Bentonite Chips	ł				Ì		j
21	_	Water @ EOB 6.0', 24hr Dry (Cave-in 5.9')	ļ		1			İ	
	_	(Out on 5.5)	ĺ		- 1				-
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numme	eiin 1	esting Services, Inc.						NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

Project:

UWSP Hyer Hall See Plan

Location: See Plan

201 Reserve St, Stevens Point, WI

Boring: SB6
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009

**Elevation:** 2/10/2009

	_	201 Reserve St, Stevens Point, WI						vation:	1090.64
Depth		Classification/Description	#	Sample	N	Rec	M	Qp	Notes
(ft)		•		Depth(ft)		(in)	3	(tsf)	
<u> </u>		Dark Brown Silty SAND w/ Organics	╅	= -1(2.5)	<del> </del>	†\ <u>``</u>	<b></b>	<del>  \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</del>	
1		(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	6	18	N.4		ļ
'	-	( OBCB. BIM ) ( OBBM. LUMIN BAIRD)	'	1 - 2.3	0	10	M	1	
	_	81+	1		ļ			<u> </u>	
2	-	Dark Brown Fine SAND, Little Silt			1				
	-	(Fill) (USCS: SP-SM) (USDA: Loamy Sand)	1		1			•	
3	_	2.51						1	
		( Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	8	18	М		
4		( Diack I cat Dayor 2.5 - 5.0 )	1 2	3.5 - 5	0	10	171		
+	-	X 1 4 75 TO 3 4 C(4 ) 175							
_	-	Light Brown F - M SAND	1		1				
5	-	(USCS: SP)(USDA: Sand)			l				
	-	5.5'			Í				
6	-		3	6 - 7.5	4	18	S		
j	-				'				
7		Light Brown Fine SAND							
'					l				
		(USCS: SP)(USDA: Sand)							
8	-	8.0'							İ
	-				1				
9	-		4	9 - 10.5	24	18	S		
	_						·		
10		Grayish Brown F - M Silty SAND							
'`		(USCS: SM) (USDA: Sandy Loam)							
,,	-			:					
11	-	( Probably Weathered From Granite )							
	- :								
12	-								
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13	_	13.0'					İ		
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14			_	14 - 15.5	20	10	s		
14	-		5	14 - 15.5	32	18	)	2.0	i
							1		
15	-	Brown Sandy CLAY					Ì		
	-	(USCS: CL) (USDA: Clay Loam)							
16	-	(Probable Weathered Granite Bedrock)							j
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	-	Brown/Red/White SAND and GRAVEL	6	18.5 - 20	43	18	S		Į
19	-	( USCS: GP ) ( USDA: Gravelly Sand )		Ì	•		1		1
		(Probable Weathered Granite Bedrock)						l	į
20	_	E.O.B. 20.0'				- 1		l	
	_	Borehole Filled with Bentonite Chips					]	ľ	ļ
21	_	Water @ EOB 6.0', 24hr Dry (Cave-in 5.4')					- 1	ľ	
-1	ĺ	water @ EOE 0.0, 24111 Dry (Cave-III 3.4)		İ			ı		
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Numme	Jin T	esting Services, Inc.						NTCH	142 80

Boring By: Nummelin Testing Services, Inc.

Project:

UWSP Hyer Hall See Plan 201 Reserve St. St

Location:

Boring: SB7
Auger: HSA
Page: 1 of 1
Drillers: NH / MA
Date: 2/16/2009
Elevation: 1090 84

		201 Reserve St, Stevens Point, WI Elevation:							1090.84
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	1	Qp (tsf)	Notes
1	-	Dark Brown Silty SAND w/ Organics (USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	4	18	М		
2	-	Dark Brown Fine SAND, Little Silt (Fill) (USCS: SP-SM) (USDA: Loamy Sand)							
3	-	(Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	6	18	M		
4	-	Light Brown F - M SAND					'''		
5	-	(USCS: SP)(USDA: Sand)							
6		6.0'	3	6 - 7.5	7	18	S		
7	-								
8	-	Light Brown Fine SAND	4	9 - 10,5	5	18	S		
10	-	(USCS: SP)(USDA: Sand)	4	9 ~ 10.3	J	18	ာ		
11	-								
12	- -	12.0'							
13	-								
14	-	Brown CLAY ( USCS: CH ) ( USDA: Clay )	5	14 - 15.5	4	18	s	1.0	
15	1 1	( Probably Weathered From Granite )							
16	-	16.5'							
17									
18	-	Brown/Red/Orange SAND and GRAVEL (USCS: SP)(USDA: Gravelly Sand)	6	18.5 - 20	42	18	S		
19 20	-	( Probable Weathered Granite Bedrock ) E.O.B. 20.0'							
21	-	Borehole Filled with Bentonite Chips Water @ EOB 6.0', 24hr 6.1'							
22	-			THE PROPERTY OF THE PROPERTY O				İ	
23	-								
24	-								
25	-	Toolship Company						KWKC 11	
ivumme	ain J	Testing Services, Inc.						NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

Project: Location:

UWSP Hyer Hall See Plan

201 Reserve St, Stevens Point, WI

SB8 **HSA** 1 of 1

Boring: Auger: Page: Drillers: Date:

NH/MA 2/16/2009

Elevation: 1090.44

		201 Reserve St, Stevens Point, WI						vation:	1090.44
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	М	Qp (tsf)	Notes
		Dark Brown Silty SAND w/ Organics	┼─		<del>                                     </del>	(/	<del> </del>	(652)	
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	9	18	М		
1	_	10"	1	' ~	´	1.0	'''		
2	_	Dark Brown Fine SAND, Little Silt							
		(Fill) (USCS: SP-SM) (USDA: Loamy Sand)				İ			
3	_	2.5'				ļ		<u> </u>	
		( Black Peat Layer 2.5' ~ 3.0')	2	3.5 - 5	11	18	М		
4		( Diack Feat Layer 2.5 ~ 5.0 )	~	3.5 - 5	11	10	IVI		
"		Light Brown F - M SAND							
5	-								
,	-	(USCS: SP)(USDA: Sand)				1			
	_	6.0'	ا ا	( 7 (		1.0	~		
6	-	0.U	3	6 - 7.5	9	18	S		
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7	_								
_	-								
8	-								
	-		.		_		_		
9	-	T 1 1 / Th. Th. OLAB TES	4	9 - 10.5	3	18	S		
,,		Light Brown Fine SAND							
10	-	(USCS: SP)(USDA: Sand)							
	-								
11	-					Ì			
	-								
12	-								
	-								
13		13.0' <b></b> -							
	-								
14			5	14 - 15.5	8	18	S	3.0	
	-								
15	-	Brownish Red CLAY					l		
	-	(USCS: CH) (USDA: Clay)	1					l	
16		(Probably Weathered From Granite)						ĺ	
	-	,					ļ		
17							l		
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18	-	18.0'						l	
	-	Brown/Red/Orange SAND and GRAVEL	6	18.5 - 20	31	18	S		
19	-	( USCS: SP ) ( USDA: Gravelly Sand )		-			I		
		(Probable Weathered Granite Bedrock)		ļ				I	
20	-	E.O.B. 20.0'						l	
1	_	Borehole Filled with Bentonite Chips	[	ŀ	İ			ļ	
21	~	Water @ EOB 6.0', 24hr Dry (Cave-in 5.4')							
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	lin T	esting Services, Inc.		L				NTS#	142.80
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Boring By: Nummelin Testing Services, Inc.

Project:

**UWSP Hyer Hall** 

See Plan (Temporary Well Installed Here) 201 Reserve St, Stevens Point, WI Location:

Boring: Auger:

SB9 **HSA** 1 of 1

Page: Drillers: NH/MA Date: 2/16/2009

Elevation: 1089 64

		201 Reserve St, Stevens Point, WI						vation:	1089.64
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)		Qp (tsf)	Notes
		Dark Brown Silty SAND w/ Organics	<del> </del>	Depth(11)	ļ	(111)	<del> </del> -	(191)	
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	6	18	М		
	-	10"	İ						
2	-	Dark Brown Fine SAND, Little Silt							
3	_	(Fill) (USCS: SP-SM) (USDA: Loamy Sand)							
)		(Black Peat Layer 2.5' - 3.0')	2	3.5 - 5	5	18	M		
4	_	( Diack I cat Layer 2.5 - 5.0 )	4	ر - د.د	,	10	IVI		
	_								
5			ļ						
							:		
6	-		3	6 - 7.5	3	18	S		
	-								
7	-								
	-	Light Brown Fine SAND			ļ				
8		(USCS: SP)(USDA: Sand)							
9	-		, l	0 105	7	1.0			
9	-		4	9 - 10.5	/	18	S		
10	_								
'`									
11	_								
	-								
12	-								
	-			:			i		
13	-	13.0'				İ			
1, 1	-		_						
14	-		5	14 - 15.5	9	18	S	1.5	
15	_	Brown CLAY					İ		
',	_	Little to Some Sand							
16	_	(USCS: CH) (USDA: Clay)						ı	
	-	( Probably Weathered From Granite )				]		1	ĺ
17	-						i		
	-						l	Į	
18	-	18.0 <sup>t</sup>				İ	j	I	
,,	~ [	Brown CLAY, Little to Some Sand	6	18.5 - 20	38	18	S	İ	
19		(USCS: CH) (USDA: Clay)						ĺ	
20	-	( Probable Weathered Granite Bedrock )					- 1	ļ	
20	_ [	E.O.B. 20.0' Borehole Filled with Bentonite Chips			ļ			ļ	
21	_ [	(Water @ EOB 5.0', 24hrs 5.0', 72hrs 5.0')							
~ 1	_	( " atto to both 5.0, 271115 5.0, 121115 5.0)			l	j		j	
22	_				Ì		I		
	-				Ì				
23	-								
	-				Į				
24	-		ļ		]		ļ		
25	-		-				1		
	12	esting Services, Inc.						NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

Project:

UWSP Hyer Hall See Plan

Location: See Plan

201 Reserve St, Stevens Point, WI

Boring: Auger:

SB10 HSA I of 1

Page: Drillers: Date:

NH / MA 2/17/2009

**Elevation:** 1091.54

		201 Reserve St, Stevens Point, WI						vation:	1091.54
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)		Qp (tsf)	Notes
<del></del>		10" of Dark Brown Silty SAND w/ Organics	· ·	· · · · · · · · · · · · · · · · · · ·	1	<u> </u>	<del>                                     </del>	<del>1 `                                   </del>	
1	-	(USCS: SM) (USDA: Loamy Sand)	1	1 - 2.5	8	18	M		
	-	10"						İ	
2	-								
		Brown to Dark Brown Fine SAND, Little Silt			Ì	ļ			
3	-	( Probable Fill )							
		(USCS: SP-SM) (USDA: Loamy Sand)	2	3.5 - 5	4	18	M	<u> </u>	
4	-	4.5'					İ		
	-	(Black Peat Layer 4.5' - 4.7')	•						
5	-	Brown F - M SAND					ŀ		
	-	( USCS: SP ) ( USDA: Sand )							
6	-	5.5' <u></u>	3	6 - 7.5	5	18	S		
	-		•						
7	-	( Water @ 7.0', 24hrs - 6.9')			1				
	-				ļ				
8	-								
	-								
9	-	Light Brown Fine SAND	4	9 - 10.5	6	18	S		
	NC .	(USCS: SP)(USDA: Sand)				l			
10	-					ļ			
11	-								
	-							<u> </u>	
12	-								
	-	10.01							
13	-	13.0'							
1,			ا ہر ا	14 15 5	١,	١,,		1	
14	-		5	14 - 15.5	4	18	S	1.0	
1.5	-								
15	-								
16	-								
10	-								
17	_								
- 17	-								
18	_	Brown Sandy CLAY							
10	_	( USCS: CH ) ( USDA: Clay Loam )							
19	_	( Probably Weathered From Granite )	6	19 - 20.5	8	18	S	2.0	
'	_	(1100aoij woamoiou 110m Oramo)		17 20.5	U	ا کر	J	۵,۰۰	
20	_ [								
	_							[	
21	_								
	_								
22	_								
	- 1								
23	_								
	-		7	23.5 - 25	25	18	S		
25	-	E.O.B. 25.0'							
	-	Borehole Filled with Bentonite Chips							
	-	Water @ EOB 7.0', 24hrs 6.9'							
Numme	lin I	Testing Services, Inc.	·L		***************************************	······		NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

UWSP Hyer Hall Moved 21'E - See Plan 201 Reserve St, Stevens Point, WI Project: Location:

Boring: Auger: **SB11 HSA** Page: Drillers: 1 of 1 NH/MA Date: 2/17/2009

Locan	vii.	201 Reserve St, Stevens Point, WI					Dai Ele	e: vation:	1090.44
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M		Notes
1	-	Brown F - M SAND (Fill) (USCS: SP)(USDA: Sand)	1	1 - 2.5	7	18	M	(65,7	
2	-	Brown F - M SAND, Some Silt (Fill) (USCS: SM) (USDA: Loamy Sand)							
3	-	3.0'							
4	- - -	Brown to Black Silty SAND Trace Gravel, Few Wood Pieces ( Poss Fill ) ( USCS: SM ) ( USDA: Loamy Sand )	2	3.5 - 5	4	18	М		
5	-	5.0'							
6	-	( Water@EOB 6.0', 24hrs - caved@5.6' & Dry ) Light Brown F - M SAND	3	6 - 7.5	6	18	S		:
7	-	(USCS: SP) (USDA: Sand)							
8	-	0.51							
9	-	8.5'	4	9 - 10.5	7	18	s		
10	_								
11	- -								
12	_	Light Brown Fine SAND							
13	- -	(USCS: SP)(USDA: Sand)							
14	-		5	14 - 15.5	12	18	S		
15	-			, ,		1	_		
16	1	16.0'							
17									
18	- -								
19	-		6	19 - 20.5	8	18	S	1.25	
20	-	Brown/Green Sandy CLAY	١	19 * 40.3	O	10	13	1.43	
	-	( USCS: CH ) ( USDA: Clay Loam ) ( Probably Weathered From Granite )							
21	-								
22	-								
23	-		_	22.5		, ,	~		
25	-	E.O.B. 25.0'	7	23.5 - 25	21	18	S		
	-	Borehole Filled with Bentonite Chips Water @ EOB 6.0', 24hrs Dry (Cave-in 5.6')				أ			
Numme	elin I	esting Services, Inc.						NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

Project: Location:

UWSP Hyer Hall See Plan

201 Reserve St, Stevens Point, WI

Boring: Auger: Page: Drillers: **SB12 HSA** 1 of 1

NH/MA Date: 2/17/2009 Elevation: 1089.39

		201 Reserve St, Stevens Point, WI						zation:	1089.39
Depth		Classification/Description	#	Sample	N	Rec	M	Qp	Notes
(ft)	Ī			Depth(ft)		(in)		(tsf)	
<del> </del>	-	Dark Brown Silty SAND w/ Organics	+	<u> </u>	<del> </del>	<del>  `                                   </del>	<b></b>	<del>  `                                   </del>	
] ]		(Fill) (USCS: SM) (USDA: Sandy Loam)	1	1 - 2.5	4	18	М		
1	_	8"	1 "	, 2	"	1.0	^**		
2	"	Black PEAT			1	1			
4	-		1						
1 ~ :		( USCS: Pt ) ( USDA: Muck )							
3	-	3.0'			]				
	-		2	3.5 - 5	14	18	M		
4	-	Light Brown F - M SAND							
	_	(USCS: SP)(USDA: Sand)				l			
5	_	, , , , , , , , , , , , , , , , , , , ,			1				
		( Water@EOB 6.0', 24hrs - caved@4.7' & Dry )	1						
6		6.0'	3	6 - 7.5	6	18	S		
'	1	Wanner A'A wenner	ادا	0-7.3	0	10	ာ		
_	-								
7	-								
] [	-								
8	-								
	-			•					
9	-		4	9 - 10.5	14	18	S		
	-								
10									
'					li				
11		Light Drover Ding CANITY							
11	-	Light Brown Fine SAND							
	-	(USCS: SP)(USDA: Sand)							ļ
12									
	-								
13	-						•		•
	_								j
14	_		5	14 - 15.5	11	18	s		ļ
				11 10,0	* *	``	~		-
15	_						ļ		
13							I		
,	-								
16	-						į		ļ
	-								ļ
17	-	**************************************							]
	-						Į		1
18	-	Brown GRAVEL, Some Sand						ł	
	_	( USCS: GP ) ( USDA: Gravelly Sand )	6	18.5 - 20	22	18	S		İ
19	_	(Probably Weathered From Granite)					~		
''	_	( 1100aoij 110aaioiou 110m Giamic)			İ				
20		E.O.B. 20.0'						I	-
40	-					l	ŀ	1	
] ,	-	Borehole Filled with Bentonite Chips						j	
21		Water @ EOB 6.0', 24hrs Dry (Cave-in 4.7')					- 1	İ	ŀ
	-						1		ţ
22	-						ı		
	-						İ		1
23	_	•		İ	l	ļ		į	
	_			ŀ		1		ı	İ
24						1	ł	l	
44	-					ı	1	ł	
2.	-								j
25							l		
Numme	lin T	osting Services Inc						NTCH	142 80

Boring By: Nummelin Testing Services, Inc.

Project: Location:

**UWSP Hyer Hall** 

See Plan 201 Reserve St. Stevens Point, WI

Boring: Auger: Page:

**SB13 HSA** 1 of 1

Drillers: NH/MA Date: 2/17/2009 Elevation: 1090.24

Depth   Classification/Description			201 Reserve St, Stevens Point, WI						ation:	1090.24
Chi	Depth		Classification/Description	#	Sample	N	Rec	M	Qp	Notes
Cuscs: sp   Cusda   Sand					Depth(ft)		(in)		(tsf)	
Srown to Dark Brown F - M SAND   Little Gravel, Cobbles (Fill)   (USCS: SP) (USDA: Loamy Sand)   2   3.5 - 5   6   18   M		-								
Second Color   Seco	1		( USCS: SP ) ( USDA: Sand )	1	1 - 2.5	21	18	M		
Little Gravel, Cobbles (Fill) (USCS: SP) (USDA: Loamy Sand)		-								w/ Spoon
- Little Gravel, Cobbles (Fill) (USCS: SP) (USDA: Loamy Sand) - 3.0'	2	-	Brown to Dark Brown F - M SAND							
3 - (USCS: SP) (USDA: Loamy Sand) 4 - Light Brown F - M SAND (USCS: SP) (USDA: Sand) 5 - (USCS: SP) (USDA: Sand) 6 6.0' 3 6 - 7.5 6 18 S  7		-	Little Gravel, Cobbles (Fill)							
Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown Fine SAND (USCS: SP) (USDA: Sand)  Light Brown Fine SAND (USCS: SP) (USDA: Sand)  Light Brown Fine SAND (USCS: SP) (USDA: Sand)  Light Brown Fine SAND (USCS: SP) (USDA: Sand)  Light Brown Fine SAND (USCS: SP) (USDA: Sand)  Light Brown Fine SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USCS: SP) (USCS: SP) (USDA: Sand)  Light Brown F - M SAND (USCS: SP) (USC	3	-	(USCS: SP) (USDA: Loamy Sand)							
Light Brown F - M SAND (USCS: SP) (USDA: Sand)	i i	-		2	3.5 - 5	6	18	M		
Light Brown F - M SAND (USCS: SP) (USDA: Sand)	4									
Solution   Continue		-	Light Brown F - M SAND							
6	5	-								
7 - 8 - 9 - 10.5 7 18 S  10 - 11 - 12 - Light Brown Fine SAND (USCS: SP) (USDA: Sand)  14 - 15.5 3 18 S  15 - 16 - 17 - 18 - 19 - 18 - 19 - 19 - 19 - 19 - 19		-	, , , , , , , , , , , , , , , , , , , ,			1				
7 - 8 - 9 - 4 9-10.5 7 18 S  10 - 11 - 12 - Light Brown Fine SAND (USCS: SP) (USDA: Sand)  14 - 5 15 - 16 - 17 - 18 - 18 - 19 - 18 - 18	6		6,0'	3	6 - 7.5	6	18	S		
8		-								
8	7					1				
9										
9	8	_								
10 - 11 - 11 - 12 - 13 - 14 - 15 - 16 - 17 - 18 - 19 - 20 - 21 - 22 - Gray/Green/Orange CLAY (USCS: CH) (USDA: Clay) (Probable Weathered Granite Bedrock)  23 - 25 - Water @ BOB 5.8', 24hr 5.8' -  Water @ BOB 5.8', 24hr 5.8' -  Light Brown Fine SAND (USCS: SP) (USDA: Sand)  5 14 - 15.5 3 18 S  19 - 10 - 11 - 12 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 18 - 19 - 10 - 10 - 10 - 10 - 10 - 10 - 10 - 10		_		1						
10 - 11 - 11 - 12 - 13 - 14 - 15 - 16 - 17 - 18 - 19 - 20 - 21 - 22 - Gray/Green/Orange CLAY (USCS: CH) (USDA: Clay) (Probable Weathered Granite Bedrock)  23 - 25 - Water @ BOB 5.8', 24hr 5.8' -  Water @ BOB 5.8', 24hr 5.8' -  Light Brown Fine SAND (USCS: SP) (USDA: Sand)  5 14 - 15.5 3 18 S  19 - 10 - 11 - 12 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 14 - 15.5 3 18 S  19 - 10 - 10 - 10 - 10 - 10 - 10 - 10 - 10	9	_		4	9 - 10.5	7	18	S		
11 - Light Brown Fine SAND 13 - (USCS: SP) (USDA: Sand) 14 - 5 14 - 15.5 3 18 S 15 - 6 19 - 20.5 9 18 S 20 - 7 21 - 6 Gray/Green/Orange CLAY 22 - Gray/Green/Orange CLAY 22 - Gray/Green/Orange CLAY 23 - 7 23.5 - 25 34 4 S 25 - Water @ EOB 5.8', 24hr 5.8' - 7 23.5 - 25 34 4 S		-								
11 - Light Brown Fine SAND 13 - (USCS: SP) (USDA: Sand) 14 - 5 14 - 15.5 3 18 S 15 - 6 19 - 20.5 9 18 S 20 - 7 21 - 6 Gray/Green/Orange CLAY 22 - Gray/Green/Orange CLAY 22 - Gray/Green/Orange CLAY 23 - 7 23.5 - 25 34 4 S 25 - Water @ EOB 5.8', 24hr 5.8' - 7 23.5 - 25 34 4 S	10	_								
12 - Light Brown Fine SAND (13 - (USCS: SP) (USDA: Sand)  14 - 5 5 14-15.5 3 18 S  15 - 6 7 7 18 8 - 7 7 23.5-25 34 4 S  20 - 7 7 23.5-25 34 4 S  21 - 8 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		-								
12 - Light Brown Fine SAND (13 - (USCS: SP) (USDA: Sand)  14 - 5 5 14-15.5 3 18 S  15 - 6 7 7 18 8 - 7 7 23.5-25 34 4 S  20 - 7 7 23.5-25 34 4 S  21 - 8 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	11									
Light Brown Fine SAND (USCS: SP)(USDA: Sand)  14 -										
Light Brown Fine SAND (USCS: SP)(USDA: Sand)  14 -	12	-								
13 - (USCS: SP) (USDA: Sand)  14 - 5 5 14-15.5 3 18 S  15 - 6 19-20.5 9 18 S  20 - 6 19-20.5 9 18 S  20 - 7 23.5-25 34 4 S  23 - 7 23.5-25 34 4 S		-	Light Brown Fine SAND							
14 -	13	-	(USCS: SP) (USDA: Sand)							
15 - 16 - 17 - 18 - 19 - 20 - 21 - Gray/Green/Orange CLAY (USCS: CH) (USDA: Clay) (Probable Weathered Granite Bedrock)  23 - 25 - Borehole Filled with Bentonite Chips Water @ EOB 5.8', 24hr 5.8'  15 - 16 - 17 - 18 - 19 - 10 - 10 - 11 - 12 - 13 - 14 - 15 - 16 - 19 - 20.5 9 18 S  18 - 19 - 20.5 9 18 S  19 - 20.5 9 18 S  19 - 20.5 9 18 S  10 - 20.5 9 18 S  10 - 20.5 9 18 S		-	, , , , , ,							
15	14	_		5	14 - 15.5	3	18	S		
16										
16	15		·							
17 - 18 - 19 - 20 - 21 - Gray/Green/Orange CLAY 22 - (USCS: CH) (USDA: Clay) (Probable Weathered Granite Bedrock)  23 - 25 - Borehole Filled with Bentonite Chips Water @ EOB 5.8', 24hr 5.8'  18			,							
17 - 18 - 19 - 20 - 21 - Gray/Green/Orange CLAY 22 - (USCS: CH) (USDA: Clay) (Probable Weathered Granite Bedrock)  23 - 25 - Borehole Filled with Bentonite Chips Water @ EOB 5.8', 24hr 5.8'  18	16	-								
18 -										
18 -	17	_				1				
19 - 6 19 - 20.5 9 18 S  20 20.3' 21 - Gray/Green/Orange CLAY  22 - (USCS: CH) (USDA: Clay)		_		] .						
19 - 6 19 - 20.5 9 18 S  20 20.3' 21 - Gray/Green/Orange CLAY  22 - (USCS: CH) (USDA: Clay)	18									
20		-					•			
20	19	-		6	19 - 20.5	9	18	S		
21		-					ŀ			
21	20	-								
Caray/Green/Orange CLAY		-	20.3 <sup>t</sup>							]
Caray/Green/Orange CLAY	21	-								
22 - (USCS: CH) (USDA: Clay) - (Probable Weathered Granite Bedrock)  23 - 7 23.5 - 25 34 4 S  25		-					1			
23	22	-	(USCS: CH) (USDA: Clay)							1
E.O.B. 25.0'		-	(Probable Weathered Granite Bedrock)							
E.O.B. 25.0'	23	-	,							
Borehole Filled with Bentonite Chips Water @ EOB 5.8', 24hr 5.8'		-		7	23.5 - 25	34	4	S		
- Water @ EOB 5.8', 24hr 5.8'	25	-								
- Water @ EOB 5.8', 24hr 5.8'		-	Borehole Filled with Bentonite Chips							
Nummelin Testing Services, Inc. NTS # 142.80		-	Water @ EOB 5.8', 24hr 5.8'							
	Numm	elin '	Testing Services, Inc.						NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

Project: Location: UWSP Hyer Hall

Moved 5'W - See Plan 201 Reserve St, Stevens Point, WI Boring: Auger: Page:

SB14 HSA 1 of 1 NH/MA

Drillers: NH/MA
Date: 2/18/2009
Elevation: 1090.24

		201 Reserve St, Stevens Point, WI						ation:	1089.34
Depth (ft)		Classification/Description	#	Sample Depth(ft)	N	Rec (in)	M	Qp (tsf)	Notes
(**)	_	Dark Brown Silty SAND w/ Organics	<del> </del>	Depin(It)	ļ	\ <u>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</u>	ļ	(101)	·
1	-	(Fill) (USCS: SM) (USDA: Sandy Loam)	1	1 - 2.5	3	4	М		
2	_	Black PEAT (USCS: Pt) (USDA: Muck)							
3	-	(Concrete@1.0') (Moved 5'W & Drilled Again)							
4	-	Light Brown F - M SAND	2	3.5 - 5	10	18	М		
	-	(USCS: SP)(USDA: Sand)							
5	_								
6	-	6.0'	3	6 - 7.5	8	18	S		
7	-								
8	_								
9	-		4	9 - 10.5	5	18	s		
10	-								
11	- -								
12	-	Light Brown Fine SAND							
13	-	(USCS: SP)(USDA: Sand)						İ	
14			5	14 - 15.5	5	18	s		
	-		3	14 - 15.5	, ,	18	8		
15	-								
16	- -						ĺ		
17	-			:					
18	-	18.0' Gray CLAY, Some Sand, Some Gravel	6	18.5 - 20	23	18	s	1 5	
19	-	(USCS: CH) (USDA: Clay Loam)	U	10.3 - 20	23	10	3	1.5	
20	-	( Probably Weathered From Granite ) E.O.B. 20.0'							
21	-	Borehole Filled with Bentonite ChipsWater @ EOB 5.5', 24hr Dry (Cave-in 4.9')							
22	-	- · · · · · · · · · · · · · · · · · · ·							
23	-					į			
	-								
24	-								
25	-	esting Services, Inc.						NTS#	142.80

Boring By: Nummelin Testing Services, Inc.

UWSP Hyer Hall See Plan Project: Location:

Boring: **SB15** Auger: **HSA** Page: 1 of 1 Drillers: NH/MA 2/18/2009 Date:

Location:	201 Reserve St, Stevens Point, WI					Dat	e: vation:	1088.19
Depth	Classification/Description	T #	Sample	ΙN	Rec		Qp	Notes
(ft)	Causimounion Description	"	Depth(ft)	``	(in)		(tsf)	140108
-	Dark Brown Silty SAND w/ Organics	<del>                                     </del>	F ()	·	1	<b> </b>	\(\cdot \cdot	
1 1 -	(USCS: SM) (USDA: Sandy Loam)	1	1 - 2.5	5	18	М		
-	8 <sup>11</sup>	1						
2 -	Brown Fine SAND, Little Silt (Fill)					l		
-	(USCS: SP-SM) (USDA: Loamy Sand)		į					
3 -	(Black Organic Silt 2.5' - 2.7')		25.5		1,0			
,   "	4.0'	2	3.5 - 5	9	18	S		
4 -	4.U							
5 -	Light Brown F - M SAND							
-	(USCS: SP)(USDA: Sand)							
6 -	6.0'	3	6 - 7.5	4	18	S		
-								j
7 -								
-								
8 -							]	
9 -			9 - 10.5	4	18	s		
] ] _		4	9-10.3	4	10	ာ		
10 -								
'   -								
11 -								
-				1				ĺ
12 -								
-								
13 -	Light Danner Fire CANTA							
14 -	Light Brown Fine SAND (USCS: SP) (USDA: Sand)	5	14 - 15.5	6	18	S		
'   _	(OSCS. SI ) (OSDA. Salid)		14-15,5		10	נו		
15 -		:						
-								
16 -								
_   -								
17 -								
18 -								
10   _								
19 -		6	19 - 20.5	12	18	S		
-						~		
20 -								
-								
21 -								
-	22.01							
22 -	23.0'							
23 -	Brown/Red/Orange Sandy CLAY							
23 -	( USCS: CH ) ( USDA: Clay Loam ) ( Probably Weathered From Granite )	7	23.5 - 25	24	18	S		
25 -	E.O.B. 25.0'	'	נוש - ט.פע	<i>4</i> -17	"0	انا		
, 1		1 1			1 1			}
-	Borehole Filled with Bentonite Chips	1 1			[ 1		!	1
-	Borehole Filled with Bentonite Chips Water @ EOB 4.0', 24hr 4.0' Testing Services, Inc.						NTS#	142.80

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page:

Route To:  Drinking Water V	Vatershed W	/ater [*** \	Naste Man	agement	Remediation	on/Redevelo	pment f	Other:		
1. General Information		· · · · · · · · · · · · · · · · · · ·			·	/ Owner Info				
Boring Number	DNR Well II	) No	County		Facility Name			<del></del>		·····
1	Ditti tion in		Portage		UWSP Hy					
Common Well Name	L.,		· · · · · · · · · · · · · · · · · · ·	# (if applic.)	Facility ID		License/Perm	it No.	City, Vill	age, or Town
				. (	142.80					s Point
1/4 / 1/4 1/4	Section		Township	Range	Street Addres	ss of Well				
			N	FFW	201 Reser	ve St				
Grid Location		Local	Grid Orlgin	}	Present Well	Owner		Original \	Well Owne	<del>.</del> r
Feet N	E	(estin	nated)	OR						
r s	₽W	Well	Location		Street Addres	s or Route	of Owner			
Latitude:		Longitude								
DEG MIN	SEC N	DEG	MIN	SEC I W	City			State	ZIP Cod	e
Reason For Abandonmen		·	Nell No. of Re	placement Well	1					
				,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	4. Pump. L	iner. Scree	n, Casing & S	ealing Ma	aterial	
3. Well / Drillhole / Borel	ole Informa	tion		······································		piping remov		T Yes	î No	i <b>v</b> N⁄A
		<del></del>	onstruction	Date	Liner(s) rer		<del> </del>	Yes	I No	₩ N/A
Monitoring Well		2/16/2009		Date	Screen rem			Yes	i No	₩ N/A
√ Water Well			Construction	Report is	Casing left			Yes	i No	₩ N/A
Borehole / Drillhole		1	please atta	-		off below su	rfo.co?		, and a	
Construction Type:		<u></u>	,		•	terial rise to		✓ Yes	No No	I VA □ NA
	Driven (san	dpoint)	Dug			ttle after 24				₩ N/A
Other (specify):	Divon (our	аропте	, bug		į			Yes	No	
					1	as hole reto		Yes	No No	V N/A
Formation Type					<b>£</b>	•	sed, were they own safe source?	[ Yes	ſ No	₩ N/A
	ation	Bedr	ock		<u> </u>					
	<del> </del>		1= . =.		•		ng Sealing Ma			
Total Well Depth From Gr	oundsurface	(ft.)	Casing Dia	ameter (in.)	Conducto	=			r Pipe-Pun	nped
····			ļ			and Poured	1 1	Other (ex	plain):	
Lower Drillhole Diameter (	in.)		Casing De	pth (ft.)	(Bentonit	e Chips)				
				;	Sealing Mate	rials	·	01	al Channe (	د المصادر المصادر الم
					Neat Cem		4			11lb/gal w t.)
Was Well Annular Space (	Grouted?	( Yes	No [	Unknow n		nent (concre	ito, cioat	Bentonite	e-Sand Slu Chine	шту
If yes, to what depth (feet)	?	Depth to w	unter (feet)		Concrete	a Molle and	Monitoring We		-	
in yes, to what depth (teet)	:	Deptit to v	vater (reet)		Bentonite	<b>5</b> 1.	Wolldoning VV		-Cement C	Frout
		5.8			Granular I	•	1		-Sand Slui	
		1		I	*** ***	No. Yar	ds, Sacks Sea	alant or	Mix	Ratio or
5. Material Used to Fill W	ell / Drillhoi	e		From (ft.)	To (ft.)		ume (circle o		Mu	d Weight
3/8" Bentonite Chips				Surface	6.0					
6. Comments										
Borehole collapsed @	) 6' <i>.</i>									•
7. Supervision of Work					DNR Use Only					
Name of Person or Firm D	oing Sealing	Work	Date of Ab	andonment	nent Date Received Noted By					
NTS, Inc.										
Street or Route P.O. Box 127	Number 1-7974									
City		State	ZIP Code		Signature of F	Person Doing	a Work		Date Sig	ned
Stevens Point		WI	54481		Signature of Person Doing Work Date Signed					· · · · · · · · ·

Route To:

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

☐ Drinking V	Nater	Watershed W	/ater ┌ '	Waste Man	agement	Remediation	on/Redevelo	pment	Other:	·····	_
1. General Ir	nformation					2. Facility	/ Owner Info	ormation			<b>***********</b>
Boring Numb		DNR Well II	) No.	County Portage	······································	Facility Name	e				
Common We					# (if applic.)	Facility ID 142.80		License/Perm	iit No.	City, Village, or To Stevens Point	wn
1/4 / 1/4	1/4	Section	<del></del>	<del></del>	V E E W	1	ve St			·······	
Feet	Grid Location  N  S	n F F W	(estim	l Grid Origin nated) Location	n OR	Present Well Street Addres		of Owner	Original V	Well Owner	
Latitude: DEG	MIN	SEC N	Longitude: DEG		SEC W	City	35 01 110010	UI OWITEI	State	ZIP Code	<del></del>
Reason For A	Abandonmen			Nell No. of Rε	eplacement Well		iner. Scree	en, Casing & S	ealing Ma	uterial	<del>,,,,,,,,,,</del>
3. Well / Dril	lhole / Bore	hole Informa	tion	<del></del>	***************************************		piping remov			I No ☑ N/A	
Monitorir Water W	ing Well		Original Co 2/16/2009	Construction Construction		Liner(s) ren Screen rem	moved? noved?		☐ Yes	No V N/A  No V N/A  No V N/A	i i
Borehole	e / Drillhole		1	please atta	·	Casing left		-f0		***************************************	
	d (**):	Driven (sand	dpoint)			Sealing ma Material sei If yes, w	off below su aterial rise to ettle after 24 l was hole reto	surface? hrs? opped?	i Yes ☐ Yes	No VA No NA No VA No VA	
Formation Ty  Unconso	/pe olidated Form	nation	Bedro	ock		hydrated with wa	ater from a kno	own safe source?		No V N/A	•
	······································	roundsurface	(ft.)	Casing Dia	iameter (in.)	Conducto	or Pipe-Gravi d and Poured	•		r Pipe-Pumped plain):	
Lower Drillhol	le Diameter (	in.)		Casing De	pth (ft.)	(Bentonite Sealing Mater	rials	<del></del>		2440 ()	<del></del>
Was Well Anı					Unknow n	Concrete	ment (concre	ete) Grout	Bentonite- Bentonite	,	t.)
If yes, to what	t depth (feet)	?	Depth to w 5.0			For Monitoring Bentonite Granular E	Chips Bentonite	<u> </u>	Bentonite-0 Bentonite-5	Cement Grout Sand Slurry	-
5. Material Us		/ell / Drillhol	6		From (ft.)	To (ft.)		ds, Sacks Sea ume (circle or	1	Mix Ratio or Mud Weight	
3/8" Bentor	nite Chips				Surface	5.0					
6. Comments							L				
Borehole co	ollapsed @	<u>)</u> 5'.									
7. Supervisio								DNR Use C	)nly		
Name of Pers NTS, Inc.		oing Sealing		02/17/09		Date Receiv			Noted By		
Street or Rout P.O. Box 12				Telephone (715) 34	1-7974	Comments					
<sup>City</sup> Stevens Po	oint		1 1	ZIP Code 54481		Signature of F	erson Doing	j Work		Date Signed	

Route To:

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

Crinking V	Vater   \	Vatershed W	/ater   \	Vaste Man	agement	nt Remediation/Redevelopment Other:					
1. General In	formation	· · · · · · · · · · · · · · · · · · ·				2. Facility	Owner inf	ormation			
Boring Numb	er	DNR Well II	O No.	County Portage		Facility Name					
Common We	ll Name			Gov't Lot	# (if applic.)	Facility ID 142.80		License/Perm	it No.	City, Villa Steven	ige, or Town s Point
1/4 / 1/4	1/4	Section		Township N		Street Addres 201 Reserv					
Feet	Grid Location N	E	Local (estin	Grid Origin nated)	OR	Present Well	Owner		Original V	Vell Owne	r
Latitude:	√ s	W	Well I	.ocation	· · · · · · · · · · · · · · · · · · ·	Street Addres	s or Route	of Owner			
DEG	MIN	SEC N	DEG	MIN I	SEC W	City			State	ZIP Code	·
Reason For A	lbandonmen	t	WI Unique V	Vell No. of Re	placement Well	4 Pump i	iner Scree	n, Casing & S	ealing Ma	terial	
3. Well / Drill	hole / Boret	ole Informa	tion		<del></del>		piping remo		Yes	No	₩A
0, 17017 15111	noic i boici	ioic iiiioiiiid		onstruction	Data	Liner(s) ren	⊢ No	IV N/A			
Monitorin	ng Well		2/17/2009	onstruction	Date	Screen rem			Yes Yes	l No	₩ N/A
Mater W	ell			onstruction	Report is	Casing left			Yes	- No	₩ N/A
Borehole	/ Drillhole		i	please atta	•	Casing cut		urface?			
Construction	Tyne:		1			1	terial rise to		¥ Yes	I No I No	₩ N/A N/A
i Drilled		Driven (sand	dpoint)	Dug			tile after 24		Yes	No No	₩ N/A
**					<del></del>		as hole reto		Yes	i No	i N∕A
Formation Ty	ре					If bentonite	chips were u	sed, were they	· · · · · · · · · · · · · · · · · · ·	No	₩ N/A
₩ Unconso	lidated Form	ation	☐ Bedro	ock		1		own safe source?		į 140	₩ 1₩A
						4 '		ng Sealing Ma			
Total Well De	pth From Gr	oundsurface	(ft.)	Casing Dia	ameter (in.)	Conducto Screened	r mpe-Grav Land Poured		Conductor Other (exp	•	ped
Lower Drillhol	e Diameter (	in.)		Casing De	pth (ft.)	(Bentonite Sealing Mater					
Was Well Anr					Unknow n	Neat Cem Sand Cem Concrete	ent Grout ent (concre	ete) Grout	Bentonite Bentonite	-Sand Slur Chips	1lb/gal w t.) ry
If yes, to what	depth (feet)	?	Depth to w 5.0	ater (feet)		For Monitoring Bentonite Granular B	Chips	page.	II Borehole Bentonite- Bentonite-	Cement Gr	
5. Material Us	sed to Fill W	ell / Drillhol	e		From (ft.)	To (ft.)		ds, Sacks Sea ume (circle oi		1	Ratio or Weight
3/8" Bentor	nite Chips				Surface	5.0					
····								······································	*****		
6. Comments								<del></del>	<del></del>	<u> </u>	
Borehole co		<u>)</u> 5'.		<del></del>			<del></del>				
7. Supervisio	n of Work							DNR Use C	nly		
Name of Pers		oing Sealing	Work	Date of Ab	andonment	Date Receiv	∕ed		Noted By	<del></del>	
NTS, Inc. Street or Rout				02/18/09 Telephone	1	Comments			Ĺ		
P.O. Box 12				(715) 34		Comments					
City Stevens Po	int	***************************************	State WI	ZIP Code 54481		Signature of F	erson Doing	g Work		Date Sign	ed

Route To:

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

Drinking V	Vater   \	Watershed V	/ater 🗀 ۱	Waste Man	agement	Remediation	on/Redevelo	pment	Other:		·····
1. General Ir	formation				·	2. Facility	/ Owner Inf	ormation			
Boring Numb 4	er	DNR Well II	O No.	County Portage		Facility Name					
Common We	ll Name			Gov't Lot	# (if applic.)	Facility ID 142.80		License/Perm	it No.	,	ige, or Town s Point
1/4 / 1/4	1/4	Section		Township N		Street Addres 201 Reser	ve St				
	Grid Location		7	Grid Origin		Present Well	Owner		Original V	Vell Owne	r
Feet	F N	E W	<u> </u>	Location	OR	Street Addres	s or Route	of Owner	<u> </u>		· · · · · · · · · · · · · · · · · · ·
Latitude: DEG	MIN	SEC N	Longitude DEG	: MIN	SEC W	City	***************************************	· · · · · · · · · · · · · · · · · · ·	State	ZIP Code	<del></del>
Reason For A	bandonmen	it	WI Unique \	Vell No. of Re	placement Well			<del></del>			
		· • · · · · · · · · · · · · · · · · · ·						n, Casing & S		terial	
3. Well / Dril	lhole / Borel	hole Informa	<del></del>			4	piping remo	ved?	Yes	No	₩ N/A
Monitorir	ng Well		1 7	onstruction	Date	Liner(s) ren			Yes	No	₩ N/A
☐ Water W	/ell		2/16/2009	Construction	Panort is	Screen rem			Yes	No No	V N/A V N/A
₩ Borehole	e / Drillhole		i	please atta	•	Casing left		<del> </del>		No No	
Construction    Drilled	1 [	Driven (san	dpoint)	Dug	or.	Sealing ma Material se	off below su terial rise to ttle after 24 vas hole reto	surface? hrs?	•	No No No	V NA NA V NA
Formation Ty						1 ' '		sed, were they	Yes	No	₩ N/A
Unconsc		nation	Bedr	ock		hydrated with wa	ater from a kno	own safe source?		No	₩ N/A
Total Well De	pth From Gr	oundsurface	(ft.)	Casing Dia	ameter (in.)	Conducto	r Pipe-Grav I and Poured		eriai Conductor Other (exp	-	ped
Lower Drillhol	e Diameter (	(in.)		Casing De	pth (ft.)	(Bentonite Sealing Mater					
Was Well An				l No l	Unknow n	Concrete	ent (concre	te) Grout	Bentonite Bentonite	-Sand Slui Chips	1lb/gal w t.) ry
If yes, to wha	t depth (feet)	)?	Depth to w	ater (feet)		For Monitorine Bentonite Granular I	Chips 3entonite	<u> </u>	Bentonite- Bentonite-	Cement G Sand Sium	ry
5. Material U	sed to Fill V	Vell / Drillhol	e		From (ft.)	To (ft.)		ds, Sacks S <del>e</del> a ume (circle or		1	Ratio or I Welght
3/8" Bentor	nite Chips				Surface	6.6					
							************	• • • • • • • • • • • • • • • • • • •	···		
6. Comments Borehole c		0) 6.6'.									
7. Supervisio	n of Work					1		DNR Use C	nly		<del></del>
Name of Person or Firm Doing Sealing Work Date of Abando NTS, Inc. 02/17/09						Date Recei			Noted By		
Street or Rout P.O. Box 1				Telephone (715) 34		Comments				,	
City Stevens Po	oint		State WI	ZIP Code 54481		Signature of F	Person Doing	g Work	-	Date Sigr	ned

Route To:

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

☐ Drinking \	<b>Vater</b>	Watershed V	Vater	Waste Man	agement	Remediati	ion/Redevelo	pment	Other:			
1. General l	nformation		- ' - '			2. Facility	/ Owner Inf	ormation				
Boring Numb		DNR Well I	D No.	County Portage	)	Facility Nam UWSP Hy						
Common We		~		Ì	# (if applic.)	Facility ID 142.80		License/Perm	it No.	City, Village, or To Stevens Point	wn	
1/4 / 1/4	1/4	Section		Township		Street Addre 201 Reser						
	Grid Location	n	Loca	I Grid Origin	<del></del>	Present Wel		*******	Original '	Well Owner		
Feet	∏ N ∏ S	ΓE Γw	(estir	nated) Location	OR	Street Addre	ss or Route	of Owner				
Latitude: DEG	MIN	SEC	Longitude DEG	: MIN	SEC	City			State	ZIP Code		
Reason For A	<b>.</b>	1	1	L	eplacement Well	/			State	ZIP Code		
, , ,	- IDGNOOTHIO		TVVI OINGGE	Voli 140. di 140	spiacement wen		iner Scree	n, Casing & S	ealing Ma	torial		
3. Well / Dril	lhole / Bore	hole Informa	ition			·	piping remo	······································	Yes	™ No IV N/A		
				onstruction	Date	Liner(s) rea			Yes	T No V N/A		
Monitori	-		2/16/2009			Screen ren	™ No W N/A					
₩ater W			If a Well C	Construction	Report is	Casing left	No V N/A					
Borehole	e / Drillhole		available,	please atta	nch.	Casing cut	off below su	rface?	Yes	No V N/A		
Construction	Type:					Sealing ma	aterial rise to		iv Yes	T No T N/A		
i <b>v</b> Drilled	•	Driven (san		[ Dug		Material se	ttle after 24	hrs?	Yes	T No VA		
Othe	r (specify):	<del></del>		· .	<del></del>	If yes, v	vas hole reto	pped?	Yes	□ No IV N/A		
Formation Ty	pe			•		If bentonite	chips were u	sed, were they	1	,		
₩ Unconso	olidated Form	nation	☐ Bedr	ock		hydrated with w	ater from a kno	wn safe source?	Yes	MO VA		
				<del></del>		4		ng Sealing Mat	terial			
Total Well De	epth From Gr	oundsurface	(ft.)	Casing Dia	ameter (in.)	E	or Pipe-Grav d and Poured	•	Conducto Other (ex	r Pipe-Pumped plain):		
Lower Drillhol	le Diameter (	(in.)		Casing De	epth (ft.)	(Bentonit						
					· · · · · · · · · · · · · · · · · · ·	Sealing Mate		· · · · · · · · · · · · · · · · · · ·	Clay Can	-l Cl	`	
Was Well An	nular Space	Grouted?	Yes	No	Unknow n	Neat Cem Sand Cen Concrete	ent Grout nent (concre	te) Grout	•	d Slurry (11lb/gal w t -Sand Slurry Chips	.)	
If yes, to wha	t depth (feet)	1?	Depth to w	/ater (feet)		For Monitorin Bentonite		Monitoring We		es Only: Cement Grout		
			6.0			Granular	Bentonite		Bentonite-	Sand Slurry		
5. Material U		Vell / Drillhol	6		From (ft.)	To (ft.)		ds, Sacks Sea ume (circle or		Mix Ratio or Mud Weight		
3/8" Bentor	nite Chips				Surface	6.0					_	
6. Comments	<del></del>		i				l <sub></sub>		····			
Borehole co		0 6'.										
			······									
7. Supervisio								DNR Use C				
Name of Pers NTS, Inc.		oing Sealing	Work	02/17/09		Date Recei	ved		Noted By			
Street or Rout P.O. Box 1:				Telephone (715) 34		Comments						
City Stevens Po	oint		State WI	ZIP Code 54481		Signature of F	Person Doing	j Work		Date Signed		

Street or Route

P.O. Box 127

Stevens Point

City

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

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Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between \$10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

form to the appropriate DN	IR office and	d bureau. S	ee instructi	ions for more in	nformation.				w.r., 0	Pr. P.		
Route To:												
	Vatershed W	Vater [	Waste Man	agement	Remediati	on/Redevelo	pment	Other:				
1. General Information					2. Facility	/ Owner Inf	ormation					
Boring Number	DNR Well II	D No.	County		Facility Name	9						
6			Portage		UWSP Hy	er Hall						
Common Well Name			Gov't Lot	# (if applic.)	Facility ID		License/Perm	nit No.	City, Vill	age, (	or Town	
				,	142.80 Stevens P							
1/4 / 1/4 1/4	Section		Township		Street Address of Well							
<u></u>			N	TET W	201 Reser	ve St						
Grid Location		Local	l Grid Origin		Present Well			Original \	Well Owne	ar.		
Feet N	ΓE	(estin	-	OR								
T s	i w			OI .	Street Addres	ss or Route	of Owner					
Latitude;	<del></del>	Longitude				00 01 1	or ornic.					
DEG MIN	SEC	DEG	MIN	SEC	City			State	ZIP Cod	e		
<u> </u>	N	<del></del>	<u> </u>	<u> </u>	,					_		
Reason For Abandonment		WI Unique V	Nell No. of Re	eplacement Well								
	· ·	<u> </u>			4. Pump, L	iner, Scree	n, Casing & S	ealing Ma	aterial			
3. Well / Drillhole / Boreh	ole Informa	ition			Pump and	piping remo	ved?	⊤ Yes	No	1	N/A	
Monitoring Well		Original C	onstruction	Date	Liner(s) rer	noved?		Yes	No		N/A	
<b>1</b> '		2/16/2009			Screen ren	noved?		Yes	No.		N/A	
["" Water Well		If a Well C	Construction	Report is	Casing left	in place?	™ No		N/A			
Borehole / Drillhole		available,	please atta	ich.			rface?	☐ Yes				
Construction Type:		<u> </u>			Casing cut off below surface?  Yes No  Sealing material rise to surface?  Yes No						N/A	
2	Driven (sand	dnoint)	Dug		•						N/A	
Other (specify):			) Dug			ttle after 24		Yes	No		N/A	
					If yes, was hole retopped?							
Formation Type					If bentonite chips were used, were they							
Unconsolidated Forma	₃tion	Bedro	ock		nydrated with water from a known sale source?							
					Required Met	hod of Placi	ng Sealing Ma	terial			_	
Total Well Depth From Gro	undsurface	(ft.)	Casing Dia	ameter (in.)	Conductor Pipe-Gravity Conductor Pipe-Pumped							
			}	1	Screened and Poured Other (explain):							
Lower Drillhole Diameter (in	n )		Casing De	enth (ft.)	(Bentonite Chips)							
Land Pinnion Pinnion (	•.,		Cuoning in a									
		***************************************	<u></u>		Sealing Mater		ſ	Clay Sand	d Slurry (1	HIh/a	≏lw † )	
Was Well Annular Space G	Prouted?	Yee	I No	Unknown	Neat Cem	ent Grout ent (concre	r	Bentonite		_	W 14 1.,	
vids vieli Alliidiai Opaos C	nouteu:	Yes	INO	Unknow n	Concrete	ient (concre		Bentonite		11 y		
If yes, to what depth (feet)?	)	Depth to w	vater (feet)			a Wells and	Monitoring We					
in Jose in miner makes from a		mobili to	ator (100c)		Bentonite	Chips		Bentonite-		rout		
		6.0			Granular I			Bentonite-				
		·		·			ds, Sacks Sea			Ratio		
5. Material Used to Fill We	ell / Drillhol	е		From (ft.)	To (ft.)		us, sacks sea ume (circle oi			. Katic d Wei		
3/8" Bentonite Chips				Surface	5.3		Mino (0	10,	Wice	J AAC	ym	
oro portorno orneo				04,14,00	0.0				<del> </del>			
		<del></del>							<u> </u>		······································	
~ ^ammanta				ı					<u> </u>			
6. Comments Borehole collapsed @	<u> </u>				····	·						
borenole collapsed @	. 0.0 .											
7 C	<del></del>				<del>1</del>		DND Has C	\ I				
7. Supervision of Work			<u> </u>	<del></del>	DNR Use Only							
Name of Person or Firm Do	ing Sealing	VVork		andonment	Date Receiv	ved		Noted By				
NTS, Inc.		1	02/17/09	,								

Telephone Number

(715) 341-7974

ZIP Code

54481

State

WI

Comments

Signature of Person Doing Work

Date Signed

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page

Route To:	Water	Watershed \	Water i <sup>™</sup>	Waste Mar	nagement	Remediati	ion/Redevelo	opment [	Other:			
1. General l	Information	v <del></del>		······································	<del></del>	2. Facility	/ Owner Int					
Boring Numl 7		DNR Well	D No.	County Portage	e	Facility Nam UWSP Hy	ie			•		
Common W	ell Name			Gov't Lot	t # (if applic.)	Facility ID 142.80	Addition	License/Perm	it No.	1	lage, or Town	
1/4 / 1/4	1/4	Section			Range N E W	Street Addre			***************************************			
	Grid Location	n	Loca	I Grid Origi		Present Wel			Original	Well Owne	er	
Feet	Γ N Γ S	Γ E Γ W	(estir	_	OR	Street Addre	es or Route	of Owner				
Latitude: DEG	MIN	SEC	Longitude DEG		SEC	City		o. Owner	State	IZIP Cod	Δ.	
Reason For	<b>I</b> Abandonmen		Wi Unique \	Well No. of R	W Replacement Well	4			Gtate	211 000	C	
						4. Pump,	Liner, Scree	en, Casing & S	ealing Ma	aterial		
3. Well / Dril	llhole / Bore	hole Informa				Pump and	piping remo	ved?	Yes	No	₩ N/A	
Monitori	ing Well		1	Construction	n Date	Liner(s) rea			Yes	No	₩ N/A	
j <sup>™</sup> Water V	Vell		2/16/2009		n Report is	Screen rer			Yes	∏ No	₩ N/A	
₩ Borehol	le / Drillhole		<b>{</b>	please atta	•	Casing left	in place? t off below su	····	Yes	No No	IV NA	
Construction	Type:	<del></del>				4	aterial rise to			∏ No □ No	™ N/A	
Drifte		Driven (san		j'' Dug		_	ettle after 24			No	₩ N/A	
Othe	er (specify):				<u></u>	lf yes, v	was hole reto	160	-	r No	₩ N/A	
Formation Ty	уре					1		ised, were they	∫ Yes	·	₩ NA	
Unconso	olidated Form	nation	Bedr	ock				own safe source? ing Sealing Mat		) 110	59 1W/1	
Total Well De	epth From Gr	oundsurface	(ft.)	Casing Di	iameter (in.)	Conductor Pipe-Gravity Conductor Pipe-Pumped Screened and Poured Other (explain):						
Lower Drillho	ole Diameter (	(in.)		Casing De	epth (ft.)	(Bentonite Chips)  Sealing Materials						
	nular Space				" Unknow n	Neat Cem Sand Cen Concrete	ent Grout nent (concre	ete) Grout	Bentonite Bentonite	-Sand Slu Chips		
If yes, to wha	at depth (feet)	i?	Depth to w			For Monitorin Bentonite Granular	Chips		Bentonite-	<i>les Only:</i> -Cement G -Sand Slur	Frout	
5. Material U	Ised to Fill W	Vell / Drillho		<u> </u>	From (ft.)	To (ft.)	No. Yar	ds, Sacks Seal ume (circle on	lant or	Mix	Ratio or	
3/8" Bentoi	nite Chips	<del></del>			Surface	6.2	1	unio joneio en	<del>-</del> ,	Min	d Weight	
										<u> </u>		
				<del></del>			<u></u>			<u> </u>		
6. Comments Borehole c	s collapsed @	<u> </u>			war				<del> </del>		***************************************	
DOLELIOIC C	Uliapaca w	y 0.2 .										
7. Supervisio	on of Work							DNR Use O	nly		·	
Name of Pers NTS, Inc.	son or Firm D	oing Sealing	Work	Date of Ab 02/17/09	oandonment 9	Date Recei	ved	-	Noted By			
Street or Rout P.O. Box 1.				Telephone (715) 34		Comments						
City Stevens Po	oint	<del></del>	1 1	ZIP Code 54481		Signature of F	erson Doing	g Work		Date Sign	ned	

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

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Route To:							ų.					
Drinking Water	Watershed W	Vater	Waste Man	agement	Remediation	on/Redevelo	pment j	Other:				
1. General Information					2. Facility	/ Owner Info	ormation					
Boring Number	DNR Well II	D No.	County		Facility Name							
8			Portage		UWSP Hy	er Hall	·		-			
Common Well Name			Gov't Lot	# (if applic.)	Facility ID 142.80		it No.	City, Villa Steven				
1/4 / 1/4 1/4	Section	······································	Township		Street Addres	ss of Well	I			<del></del>		
	<u> </u>		N.	1 E E W	201 Reser							
Grid Locatio			l Grid Origin	1	Present Well Owner Original Well Owner							
Feet N	E		nated)	OR								
s	₩.		Location	<del></del>	Street Address or Route of Owner							
Latitude: DEG MIN	SEC	Longitude DEG	: MIN	SEC	<u> </u>			151-1-	1315 Code		<u> </u>	
DEG NAT	) SEC		10017	J W	City			State	ZIP Code	)		
Reason For Abandonme	nt	WI Unique \	Well No. of Re	eplacement Well					<u></u>			
					4. Pump, Liner, Screen, Casing & Sealing Material							
3. Well / Drillhole / Bore	hole Informa	ition			Pump and	piping remov	/ed?	Yes	No	₩ N/A	<del>-</del>	
Monitoring Well		Original C	Construction	Date	Liner(s) rer	noved?		Yes	No	₩ N/A	١	
		2/16/2009			Screen ren	noved?		Yes	No	₩ NA		
Water Well		i	Construction	•	Casing left	in place?		Yes	No No	₩ N/A		
Borehole / Drillhole		available,	please atta	ıch.	Casing cut	off below su	rface?	Yes	No	₩ N/A	١	
Construction Type:	<u> </u>				Sealing ma	aterial rise to		✓ Yes	No	□ N/A	,	
	Driven (san		Dug		Material se	ittle after 24 l	hrs?	Yes	No	₩ N/A		
Other (specify):	<del></del>	<del></del>		, 	If yes, w	vas hole reto		Yes	. No	₩ N/A	١	
Formation Type	······································	<del></del>	- <del></del>		If bentonite	chips were us	sed, were they	•	`			
Unconsolidated Form     Unconsolidated Form	nation	Bedr	ock			•	wn safe source?	Yes	No	₩ NA	4	
<del>                                  </del>	100000	* =	00		Required Met	thod of Placi	ng Sealing Ma	terial				
Total Well Depth From G	roundsurface	(ft.)	Casing Di	ameter (in.)	Conducto				r Pipe-Pum	ped		
					Screened and Poured Other (explain):							
Lower Drillhole Diameter	(in.)	· · · · · · · · · · · · · · · · · · ·	Casing De	epth (ft.)	(Bentonit	e Chips)						
			_	• • •	Seafing Mater	riale						
	, , , , , , , , , , , , , , , , , , ,		L		Neat Cem		-	Clay San	d Slurry (1	1lb/gal w	't.)	
Was Well Annular Space	Grouted?	Yes	□ No □	Unknow n	1	nent (concre			-Sand Slur	ту		
					Concrete	·	i V	Bentonite	=			
If yes, to what depth (feet	)?	Depth to v	vater (feet)		Prom	OI :	Monitoring We		· .			
				!	Bentonite	•			Cement G			
· · · · · · · · · · · · · · · · · · ·	rii	6.0		·	Granular I				-Sand Slum			
5. Material Used to Fill \	Nell / Drillhol	ie		From (ft.)	To (ft.)		ds, Sacks Sea ume (circle or		1	Ratio or I Weight		
3/8" Bentonite Chips	i			Surface	5.3							
6. Comments												
Borehole collapsed (	@ 5.3'.								<del></del>			
7. Supervision of Work				<del></del>	T		DNR Use C	nly				
Name of Person or Firm I NTS, Inc.	Doing Sealing	Work	Date of Ab 02/17/09	oandonment	Date Recei	ved		Noted By				
Street or Route	· · · · · · · · · · · · · · · · · · ·		Telephone	Number	Comments	<del></del>	<del> </del>	L			***************************************	
P.O. Box 127			(715) 34	1-7974								
City			ZIP Code		Signature of F	erson Doing	y Work		Date Sign	ed		
Stevens Point		WI	54481						Ì			

P.O. Box 127

Stevens Point

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

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Notice: Completion of this report is required by chs. 160, 281, 283, 289, 291-293, 295 and 299, Wis Stats., and ch. NR 141, Wis. Adm. Code. In accordance with chs. 281, 289, 291-293, 295, and 299, Wis. Stats., failure to file this form may result in a forfeiture of between \$10-25,000, or imprisonment for up to one year, depending on the program and conduct involved. Personally identifiable information on this form is not intended to be used for any other purpose. Return form to the appropriate DNR office and bureau. See instructions for more information.

form to the a	appropriate D	NR office and	d bureau. S	See instruct	tions for more	information.							
Route To:													
Crinking \	Water [	Watershed V	Vater [	Waste Mar	nagement	Remediat	ion/Redevelo	ppment i	Other:		<del></del> -		
1. General I	nformation				<del></del>	2. Facility	/ Owner Inf	ormation	<del></del>		<del></del>		
Boring Numb	ber	DNR Well I	D No.	County		Facility Nam	·		***************************************				
10	)			Portage	€	UWSP Hy							
Common We	ell Name		***************************************		# (if applic.)	Facility ID		License/Perm	it No.	City, Vill	lage, or Town		
					,	142.80					ns Point		
1/4 / 1/4	1/4	Section		Township		Street Addre				10.0	IO I OIII		
<u> </u>					ALEL M	201 Resei	201 Reserve St						
	Grid Location		Loca	l Grid Origir		Present Well Owner Original Well Owner							
Feet	N	E		nated)	OR								
	√ S	Γw	☐ Well	Location		Street Address or Route of Owner							
Latitude:			Longitude			1							
DEG	MIN	SEC	DEG	MIN	SEC "	City		······································	State	ZIP Code	e		
Dageon For	Abandonmen	<u>,                                    </u>		intellible of D	eplacement Well								
Meason i o. /	Muantuoninten	H	VVI Unique (	Well No. of Re	eplacement vveii				<u> </u>				
	*** * * * * * * * * * * * * * * * * * *		<u> </u>					n, Casing & S					
3. Well / Dril	ilhole / Bore	hole Informa	7			Pump and	piping remo			No	₩ N/A		
["" Monitori	ina Well		Original C	onstruction	Date	Liner(s) rei	moved?		Yes	No	₩ N/A		
· ·	-		2/17/2009			Screen ren	₩ N/A						
Water W			If a Well C	Construction	n Report is	Casing left	i♥ NA						
Borehole	e / Drillhole		available,	please atta	ach.	1	off below su	irface?	Yes	No	₩ N/A		
Construction	Type:		<del>-l</del>			-	aterial rise to		Yes ✓ Yes		N/A N/A		
□ Drilled		Driven (sand	dpoint)	Dug			ettie after 24			No No			
* * *	r (specify):	•				1			Yes	No	₩ N/A		
						4	vas hole reto		Yes	No No	₩ N/A		
Formation Ty	•							sed, were they	Yes	No	₩ NA		
₩ Unconso	olidated Form	nation	Bedro	ock		hydrated with w	ater from a kno	own safe source?	169	1 110	} <b>∀</b> 1₩/:\		
						Required Me	thod of Placi	ng Sealing Mat	erial				
Total Well De	epth From Gr	oundsurface	(ft.)	Casing Dia	ameter (in.)	Conductor Pipe-Gravity Conductor Pipe-Pumped							
						Screened and Poured Other (explain):							
Lower Drillho	le Diameter (	(in.)		Casing De	epth (ft.)	(Bentonite Chips)							
	•	,			.hr ()	OE Mata		***************************************					
	<del>/</del>				W	Sealing Mate		1	Clav Sand	1 Slurry (1	11lb/gal w t.)		
Was Well An	nular Space	Grouted?	T Yes	r. No r.	' I beknow n	Neat Cem	ent Grout nent (concre	122	Bentonite-		- ,		
VVCC VVC	Hular Opacs	Giodica:	162	No	Unknow n	Sand Cen Concrete	nent (concre	,	Bentonite		11 <b>y</b>		
If yes, to wha	t depth (feet)	?	Depth to w	rater (feet)			a Malls and	Monitoring We		•			
··· • •	- w-F / /	•		uto, (1001)		Bentonite	Chips		<i>ii Boretiole</i> Bentonite-0		rout		
			7.0			Granular	•		Bentonite-9				
		<del></del>			<u> </u>			ls, Sacks Sea			,		
5. Material U	sed to Fill W	/ell / Drillhol	е	-	From (ft.)	To (ft.)		as, Sacks Sea: ume (circle on			Ratio or d Weight		
3/8" Bentor	nite Chips		14:11: Lai Linia (14:14:14:14:14:14:14:14:14:14:14:14:14:1		Surface	7.1					* ********		
								***************************************					
6. Comments	ŝ	<del></del>		***************************************	<u> </u>					<u> </u>	<del></del>		
Borehole co		D 7.1'.			<u></u>						<del> </del>		
		2											
7. Supervisio	n of Work			·····				DNR Use O	niv	***************************************			
Name of Pers	····	oing Sealing			andonment	Date Recei			Noted By	<del></del>			
NTS, Inc.				02/18/09		1							
Street or Rout	Ι.			Telephone	Number	Commonic	Comments						

(715) 341-7974

Signature of Person Doing Work

Date Signed

ZIP Code

54481

State

WI

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

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_			_
н	n:	ıtα	To

Drinking \	Water	Watershed \	Vater i <sup>™</sup>	Waste Mai	nagement	Remediation/Redevelopment Other:							
1. General l	nformation					2. Facility	/ Owner Inf	ormation	<del></del>	· · · · · · · · · · · · · · · · · · ·			
Boring Numb		DNR Well	ID No.	County Portage	9	Facility Nam UWSP Hy							
Common We	ell <b>N</b> ame				# (if applic.)	Facility ID 142.80		License/Perr	nit No.	4	age, or Town		
1/4 / 1/4	1/4	Section		Township	Range N E  W	Street Addre			<del></del>	10.000	IST OILL		
Feet	Grid Location	n E		al Grid Origi mated)			Present Well Owner Original Well Owner						
Latitude:	S	™ W	Longitude	Location		Street Addre							
DEG	MIN		DEG	MIN	SEC V				State	ZIP Cod	9		
Reason For /	Abandonmen	t	WI Unique	Well No. of R	leplacement Wel	4. Pump, Liner, Screen, Casing & Sealing Material							
3. Well / Dril	lhole / Borel	nole Inform	ation			Pump and	piping remov	ved?	Yes	No	₩ N/A		
[ Monitori	ng Well		Original (	Construction	n Date	Liner(s) re			Yes Yes	No No	₩ N/A		
₩ater W			If a Well	Construction	•	Screen removed? Yes No WA Casing left in place? Yes No WA							
Construction			available,	please atta	ach.	Casing cut off below surface? Yes No WA Sealing material rise to surface? Yes No N/A							
Drilled Other	d [T r (specify): _	Driven (sar	idpoint)	Dug		Material se	ettle after 24 l	hrs?	├ Yes	No	₩A		
Formation Type						.1		sed, were they	Yes	No	₩ N/A		
Ī	olidated Form	ation	∏ Bedi	rock		hydrated with w	ater from a kno	wn safe source?	Yes	No No	₩ N/A		
Total Well De	pth From Gro	oundsurface	(ft.)	Casing Di	iameter (in.)	<b>4</b>	thod of Placir or Pipe-Gravi	ng Sealing Ma ity		г Pipe-Pun	ped		
Lower Drillhol	e Diameter (i	n.)		Casing De	epth (ft.)	Screened and Poured Other (explain): (Bentonite Chips)							
·	***************************************	· · · · · · · · · · · · · · · · · · ·				Sealing Mate		· · ·	Clay San	d Shirry (1	1lb/gal w t.)		
Was Well Anı	nular Space (	Grouted?	Yes	No T	Unknow n	Neat Cem Sand Cen Concrete	ent Grout nent (concre	te) Grout		-Sand Slu	Ŧ ,		
If yes, to what	depth (feet)	?		vater (feet)		Bentonite	Chips	****	Bentonite-	Cement G			
5. Material Us	sed to FIII W	ell / Drillbo	6.0	<del></del>	From (ft.)	Granular   To (ft.)		ls, Sacks Sea		Sand Slur	y Ratio or		
3/8" Bentor							Volu	ıme (circle o	ne)	Muc	l Weight		
O/O Demoi	inc Omps			***************************************	Surface	6.6							
6. Comments	;				1								
Borehole co	ollapsed @	6.6'.									• •		
7. Supervisio	n of Work							DNR Use C	nly				
Name of Perso NTS, Inc.		oing Sealing	Work	02/18/09		nt Date Received Noted By							
Street or Route P.O. Box 12				Telephone (715) 34		Comments							
City State ZIP Code Stevens Point WI 54481						Signature of Person Doing Work Date Signed					ed		
									<del></del>				

#### Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

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Route To: Drinking Water	Watershed W	/ater   \	Waste Man	agement	Remediation	n/Redevelo	pment [	Other:			
1. General Informatio	n				2. Facility	Owner Info			······································	<del></del>	
Boring Number	DNR Well II	D No.	County		Facility Name						··· · · · · · · · · · · · · · · · · ·
12			Portage		UWSP Hy						
Common Well Name				# (if applic.)	Facility ID		License/Perm	it No.	City, Vill	age. o	r Town
				. ( app)	142.80				Stever		
1/4 / 1/4 1/4	Section	<del> </del>	Township	Range	Street Addres	s of Well	<u> </u>		10.070.		
1,-7, 1,-4				TET W	201 Reser						
Grid Loca	tion	Local	Grid Origin	<u></u>	Present Well			Original V	Vell Own	ər	
Feet	N E		nated)	OR							
l cot		,	Location	UK	Street Addres	e or Route	of Owner	l			
Latitude:		Longitude			Officet Address	is or reduce v	or Owner				
DEG MIN	I SEC	DEG	MIN	SEC	City	<del> </del>		State	ZIP Cod	e	
	1 1		<u> </u>	W	<b>1</b>						
Reason For Abandonn	nent	W/ Unique V	Vell No. of Re	placement Well				<u></u>			
					4. Pump, L	iner, Scree	n, Casing & S	ealing Ma	iterial		
3. Well / Drillhole / Bo	rehole Informa	tion			Pump and	piping remo	ved?	Yes	No	ĩ <b>ũ</b>	N/A
Admiterium Mall		Original C	onstruction	Date	Liner(s) ren	noved?		Yes	∏ No	V	N/A
Monitoring Well		2/17/2009			Screen rem	noved?		Yes	No.	<b>i</b>	N/A
☐ Water Well		If a Well C	Construction	Report is	Casing left	in place?		Yes	i No	V	N/A
😿 Borehole / Drillhole	€	available.	please atta	ch.	P	off below su			F***	V	
Construction Type:	<u></u>	1	,		_	terial rise to		Yes ✓ Yes	No		N/A N/A
	Driven (san	dnoint\	] Dug		ľ	ttle after 24			No		
Other (specify	•	•	-					Yes	No	V 1	
	· ) ·			··············	•	as hole reto	, -	Yes	No	V	WA
Formation Type						•	sed, were they	Yes	No		N/A
₩ Unconsolidated Fo	ormation	Bedr	ock		hydrated with wa	ater from a kno	own safe source?	1 169	; ,,,,	· · · · · · · · · · · · · · · · · · ·	1411
					Required Met	hod of Placi	ng Sealing Ma	terial			
Total Well Depth From	Groundsurface	(ft.)	Casing Dia	ameter (in.)	Conducto	r Pipe-Grav	ity	Conductor	г Pipe-Pur	nped	
					Screened	l and Poured	ı F	Other (exp	plain):		
Lower Drillhole Diamet	er (in.)		Casing De	pth (ft.)	(Bentonit	e Chips)					
	(,				Sealing Mater	dala					
	······································		<u> </u>		Neat Cem		[	Clay Sand	d Slurry (	11lb/ga	al w t.)
Was Well Annular Spa	ce Grouted?	Yes	I''' No I''	Unknow n	i'	ent (concre	person.	Bentonite		-	
vas von malai opa	oc orodica:	( 165	( NO )	Olikilow n	Concrete	DIR (0011010	,	Bentonite		-	
If yes, to what depth (fe	eet)?	Depth to w	rater (feet)			g Wells and	Monitoring We	ll Borehole	es Only:		
	•	l '	` '			Chips		Bentonite-		3rout	
		6.0			🦳 Granular I	3entonite		Bentonite-			
5. Material Used to Fi	ll Well / Drillhol	e		From (ft.)	To (ft.)		ds, Sacks Sea ume (circle o			Ratio	
3/8" Bentonite Chi	os		<u> </u>	Surface	4.8						Maria
		······································					······································		<b>1</b>	······	
					······						
6. Comments				<u> </u>					.L		
Borehole collapsed	@ 4.8'.									<u> </u>	
7. Supervision of Wo	·le		<del></del>				DNR Use C	niv	·····		
		Mork	Date of Ah	andonment	Date Recei	vod	DITIT GOC C	Noted By	······································		
Name of Person or Firr NTS, Inc.	n Doing Sealing	VVUIK	02/18/09		Date Recei	veu		INDIGU DY			
Street or Route			Telephone		Comments			L			
P.O. Box 127			(715) 34								
City		State	ZIP Code		Signature of F	Person Doing	g Work		Date Sig	ned	··· · · · · · · · · · · · · · · · ·
Stevens Point		WI	54481		_	·					

Route To:

## Well / Drillhole / Borehole Abandonment

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Drinking \	Water ፲ \	Natershed W	later	agement	Remediation/Redevelopment Other:							
1. General li	nformation					2. Facility	/ Owner Inf	ormation			***************************************	······································
Boring Numb	er	DNR Well II	) No.	County		Facility Name	)			<del> </del>		
13				Portage		UWSP Hy	er Hall					
Common We	ell Name			Gov't Lot	# (if applic.)	Facility ID 142.80		License/Perm	it No.	City, Villa Steven	-	
1/4 / 1/4	1/4	Section		Township	Range	Street Addres	s of Well	<del></del>	<del></del>	· <del>^</del>		
				N	E W	201 Reser	ve St					
	Grid Location		Local	Grid Origin	1	Present Well	Vell Owne	r				
Feet	N	ſ E		nated)	OR							
	T s	€W		Location		Street Addres	s or Route	of Owner				
Latitude:	3.418.1	OF C	Longitude		050			<del></del>	15	12.2.2		···········
DEG	MIN I	SEC I N	DEG	MIN	SEC W	City			State	ZIP Code	<del>}</del>	
Reason For A	Abandonmen			Vell No. of Re	eplacement Well	1			]	}		
			<u> </u>		<u> </u>	4. Pump, I.	iner, Scree	n, Casing & S	ealing Ma	iterial	***************************************	~~~~~
3. Well / Dril	lhole / Borel	nole Informa	tion		· · · · · · · · · · · · · · · · · · ·		piping remo			Ĩ No	N C	VA.
			Original C	onstruction	Date	Liner(s) ren	noved?		Yes	™ No	V	
i <sup></sup> Monitori	ng Well		2/17/2009			Screen rem			Yes	No.	V	
™ Water W	Veli		If a Well C	onstruction	Report is	Casing left		Yes	···· No	V		
Borehole	e / Drillhole		available,	please atta	ch.		off below su	uface?	Yes	No	₽ N	
Construction	Type:	<del></del>	<u> </u>	·	······································	•	terial rise to		¥ Yes	i No		VA VA
₩ Drille		Driven (sand	dpoint)	Dug		E -	ttle after 24		Yes	i No		VA.
	r (specify):						as hole reto		•		V	
Formation Ty					w-t	ł		sed, were they	Yes	No.	140 17	W.M.
_	•	4!	gover Marala			hydrated with wa	omps were u eter from a kno	own safe source?	Yes	No No	₩ I	<b>V</b> A
j <b>y</b> Unconso	olidated Form	ation	Bedr	OCK		4						
Total Mail De	epth From Gr	oundeurface	/ <del>6</del> \	Casina Dir	ameter (in.)	Conducto		ng Sealing Ma	Conductor	r Dìna Dun	mod	
TOtal Well De	spar r tota Gi	Oundounace	(11.)	Casing Die	arrieter (m.)		and Poure		Other (exp	-	hen	
Lower Deille	la Diameter /	in \		Casina Da	oth /ft \	(Bentonite		,	Other (ox	picarry.		
Lower Drillho	ie Diameter (	161. j		Casing De	pur (ii.)	Sealing Materials						<del></del> -
		···•		<u> </u>		·		ţ^	Clay Sand	d Slurry (1	1lb/qal	w t.)
Was Well An	nular Space	Grouted?	Yes	No [	Unknow n	Neat Cem	ent Grout ment (concre	garan.	-	-Sand Slu	-	,
TTO TTO IT	maiai opacc	Oroutou:	1 65	) 140 [	OUNIOWII	Concrete	en (conorc		Bentonite		•	
If yes, to wha	t depth (feet)	?	Depth to v	rater (feet)		For Monitoring Wells and Monitoring Well Boreholes Only:						
			}			Bentonite			Bentonite-			
			5.8			Granular I	Bentonite		Bentonite-	Sand Slur	ry	
5. Material U	sed to Fill W	/ell / Drillhol	е		From (ft.)	To (ft.)		ds, Sacks Sea ume (circle o			Ratio d Weig	
3/8" Bento	nite Chips				Surface	6.0						
		·										
6. Comment												
Borehole c	collapsed @	9 6.0'.										
7. Supervisio	on of Work		<del></del>					DNR Use C	Only			
Name of Pers		oing Sealing	Work	Date of Ab	andonment	Date Recei	ved		Noted By			
NTS, Inc. 02/18/09												
Street or Route Telephone Number P.O. Box 127 (715) 341-7974												
City			State	ZIP Code		Signature of F	Person Doin	g Work	,	Date Sigi	ıed	
Stevens Point WI 54481												

W

Stevens Point

54481

# Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

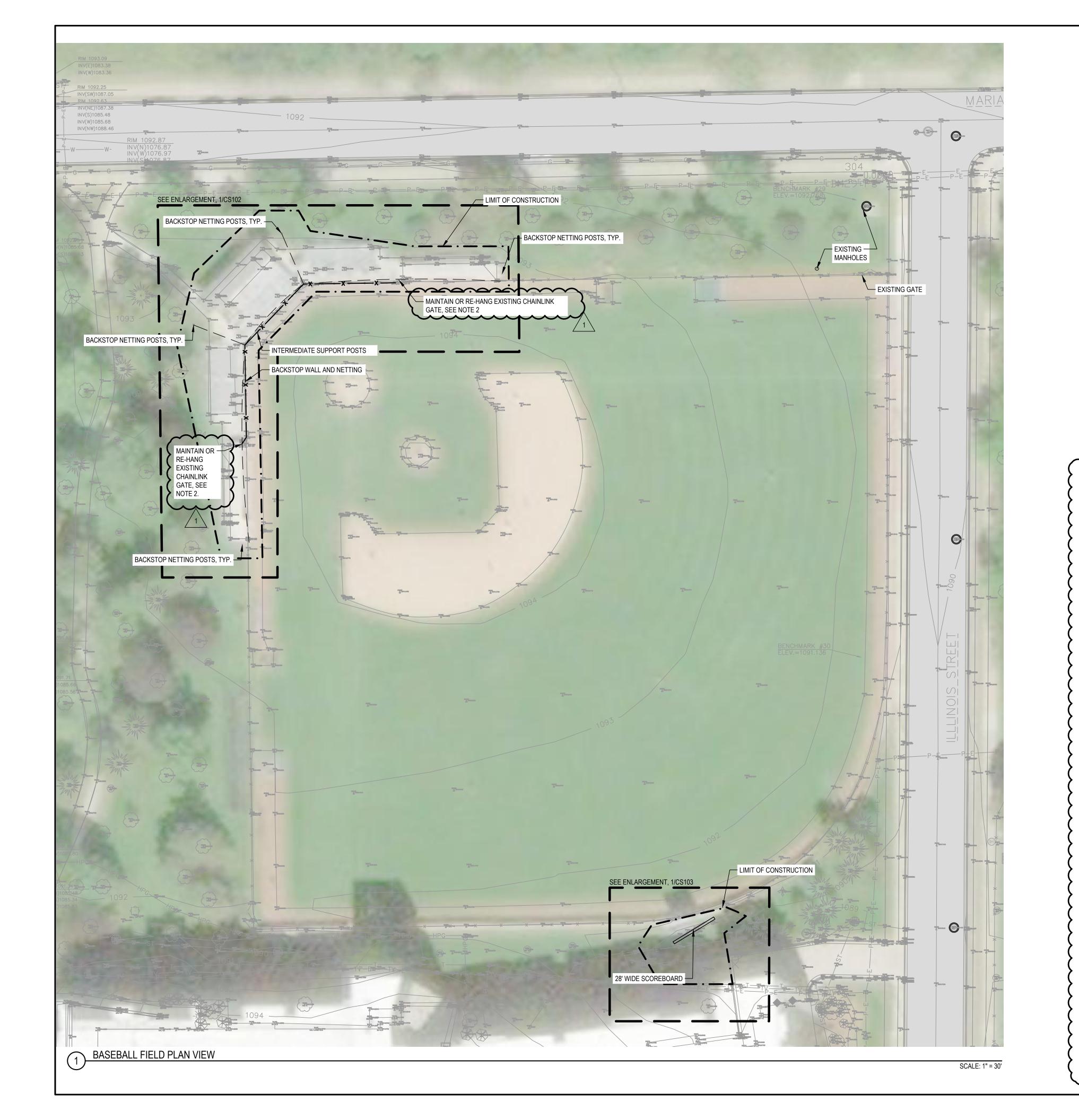
to the appropriate Divit of	nice and buleau.	See msnuci	ions to more	шошацоп.							
Route To:											
	rshed Water	Waste Mar	nagement	Remediati	on/Redevelo	pment [	Other:		····		
1. General Information		·		2. Facility	/ Owner Inf	ormation					
	R Well ID No.	County		Facility Nam							
14	***************************************	Portage		UWSP Hy	er Hall						
Common Well Name		Gov't Lot	# (if applic.)	Facility ID		License/Perm	it No.		age, or Town		
			ļ.	142.80				Stever	s Point		
1/4 / 1/4   1/4   Seci	tion	Township		Street Addre							
Cid Lastin	Towar		ALEL M								
Grid Location Feet N	touts	al Grid Origii	n	Present Well Owner Original Well Owner							
l		mated) Location	OR								
[ S	**			Street Address or Route of Owner							
Latitude: DEG MIN	Longitude SEC DEG	e: MiN	SEC	City			State	ZIP Cod			
1	N	]	L w				State	ZIP Cou	е		
Reason For Abandonment	WI Unique	Well No. of R	eplacement Well								
				4. Pump, Liner, Screen, Casing & Sealing Material							
3. Well / Drillhole / Borehole I	nformation			Pump and	piping remo	ved?	Yes	No	₩ N/A		
Monitoring Well	Original (	Construction	Date	Liner(s) rer	moved?		Yes	No	₩ N/A		
	2/18/2009	9		Screen removed? Yes No							
Water Well	If a Well	Construction	n Report is	Casing left in place? Yes ☐ No 🕡 N/A							
Borehole / Drillhole	available,	, please atta	ach.	Casing cut	off below su	ırface?	Yes	No	₩ N/A		
Construction Type:				Sealing ma	aterial rise to		Yes	□ No	N/A		
	en (sandpoint)	Dug		Material se	ttle after 24	hrs?	Yes	. No	₩ N/A		
Other (specify):				If yes, v	vas hole reto	pped?	Yes	No	₩ N/A		
Formation Type				ł		sed, were they	103				
Unconsolidated Formation	∫ Bed	rock			•	own safe source?	Yes	i No	₩ NA		
,•	,			Required Met	hod of Placi	ng Sealing Mat	erial				
Total Well Depth From Grounds	surface (ft.)	Casing Di	ameter (in.)	Conducto		-		r Pipe-Pun	med		
·	` ,		` ,	Screened	and Poured		Other (ex		,		
Lower Drillhole Diameter (in.)		Casing De	epth (ft.)	(Bentonit			· .				
(,		Journa D.	- P ( )	Caplian Maia							
**************************************		J		Sealing Mate Neat Cem		1	Clay San	d Slurry (1	l1lb/gal w t.)		
Was Well Annular Space Grout	ed? Yes	No	Unknow n		nent (concre	te) Grout	Bentonite	-Sand Slu	rry		
				Concrete		i v	Bentonite				
If yes, to what depth (feet)?	Depth to	water (feet)				Monitoring We	l Borehol	es Only:			
				Bentonite	•			Cement G			
	5.5	)		Granular				Sand Slur			
5. Material Used to Fill Well / I	Drillhole		From (ft.)	To (ft.)	•	ds, Sacks Sea ume (circle on			Ratio or		
3/8" Bentonite Chips		<del>-</del>	Surface	4.9	101	ame (on one on	~,	swei	d Weight		
					<u> </u>			· · · · · · · · · · · · · · · · · · ·			
							· ·····	<del> </del>			
6. Comments			<u> </u>	<u> </u>	<u> </u>			4			
Borehole collapsed @ 4.9	· .	· · · · · · · · · · · · · · · · · · ·									
7. Supervision of Work		·				DNR Use O	nly				
Name of Person or Firm Doing S	Sealing Work		andonment	Date Recei	ved	-	Noted By				
NTS, Inc.		02/19/09									
Street or Route		Telephone		Comments							
P.O. Box 127 City	State	(715) 34 ZIP Code		Signature of F	Jaraan Mater	v Marie		Inata O:			
ORY	JOISTE	LAIT COUR		orgridure of F	CISCII LICING	ą VVOIK		Date Sign	ı <del>c</del> ü		

# Well / Drillhole / Borehole Abandonment

Form 3300-005 (R 10/03)

Page 1

Route To:  Drinking Wat	iter [ V	Watershed W	Nater [	Waste Mar	nagement	Remediat	tion/Redevelop	nment 1	Other:			
1. General Info					agomen		y / Owner Info		O			
Boring Number		DNR Well II	D No.	County		Facility Name		// IIIQUO,			-	
15			J 110.	Portage	a	UWSP Hy						
Common Well N	Name	<del></del>	***************************************		# (if applic.)	Facility ID	•	License/Perm	nit No	Taity Vill	age, or Town	
	****				" \" "F1",	142.80	ļ	Liberious, E	KING.		age, or rown is Point	
1/4 / 1/4 1/4	4	Section	<del></del>	Township		Street Addre	ess of Well	10000	3 1 On 15			
	·· <del>···································</del>	<u> </u>			NETW	201 Reser	rve St					
Grid	id Location	·	Loca	d Grid Origin	n	Present Well			Original 1	Well Owne	ar.	
Feet	N	E		mated)	OR							
	ī s	i w		Location		Street Addre	ess or Route o	of Owner				
Latitude: DEG	MIN	SEC	Longitude DEG		PEC							
DEG	IVnew /	SEC N		MIN	SEC W	City			State	ZIP Code	3	
Reason For Aba	andonmen			Well No. of R	Replacement Well				-			
					<u> </u>		Liner, Screen	n, Casing & S	ealing Mr	aterial		
3. Well / Drillhol	ole / Borel	nole Informa	ation				piping remove		Yes	í No	₩ N/A	
	***************************************			onstruction	Date	Liner(s) rer	· · · ·		Yes	No	₩A WA	
Monitoring V			2/18/2009		*** *** !	Screen rem			Yes	i No	₩ N/A	
Water Well				Construction	n Report is		Casing left in place?					
Borehole / D	Drillhole		i	please atta			t off below sur					
Construction Typ	ne:	***************************************	<u> </u>	<del></del>			aterial rise to s		i Yes ✓ Yes	No No	₩ N/A N/A	
i <b>⊽</b> Drilled		Driven (sand	dpoint)	∫™ Dug	*	1	ettle after 24 h				V NVA	
Other (s				,	·····		was hole retop	nnod?	Yes	No		
Formation Type		***************************************				-	·	•	Yes	No	₩ N/A	
Unconsolida		W	i Bode		,	If Demonic	e chips were use vater from a know	ed, Were mey	☐ Yes	∏ No	₩ N/A	
i <b>y UNCOHS</b> ONGA	ated Forms	ation	Bedro	OCK	7		<del>· · · · · · · · · · · · · · · · · · · </del>					
Total Well Depth	- From Gr	oundeurface.	/4 \	Casina Di	iameter (in.)	**	ethod of Placin for Pipe-Gravity			~ · · · · · · · · · · · · · · · · · · ·		
10tai vven Dept	i Flom Gra	MINGOLLINGO.	(11.)	Casing Die	imeter (nr.)	Į.	or mpe-Gravity and Poured	• '. '	Conductor Other (exp	r Pipe-Pum ntain):	ped	
' awar Dritthole F	Sameter (	: \	·····	Casina De	1 L / CL \	(Bentonite		,	JUIGI (CV)	Dan).		
Lower Drillhole D	латест (п	n.)	!	Casing De		****		<del></del> ,	<del></del>			
***************************************				<u> </u>		Sealing Mater		[***	Clay San	d Chirry (1	1lb/gal w t.)	
Was Well Annula	or Space (	Crautad?	**** Vac	tim kin pur	" telesana	1	nent Grout ment (concrete	1		e-Sand Slur	_ ,	
VVIIS VVOII / TITLING	at Opace	Monteo:	Yes	No	Unknow n	Sand Cerr Concrete	ment (concrete	,	Bentonite		гу	
If yes, to what de	apth (feet)	?	Depth to w	√ater (feet)			: na Wells and N			•		
( );	har.			aro. (.	7	Bentonite	e Chips			<i>es Uniy:</i> -Cement Gr	rout	
		1	4.0		,	Granular E				-Sand Slurr		
5. Material Used	d to Fill W	/ell / Drillhol	e		From (ft.)	To (ft.)	•	ls, Sacks Seal			Ratio or	
				J			Volu	ıme (circle on	<u>.е)</u>	Mud	i Weight	
3/8" Bentonite	3 Chibs		***************************************		Surface	4.2	<b></b>			<del> </del>		
	<del></del>				<b> </b>	<u> </u>		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		<del> </del>	····	
6. Comments								<del></del>		<u></u>	<del></del>	
Borehole colla	nead (d	<del>Л Д 7) —</del>	<del></del>		***************************************						***************************************	
20101010 00114	thaca @	; 4,∠.										
7. Supervision o						<u> </u>		DNR Use O	niy			
Name of Person o	or Firm Do	oing Sealing	1	ł	pandonment	Date Receiv	ved		Noted By			
NTS, Inc.	····			02/19/09					·	·	<del></del>	
Street or Route				Telephone		Comments						
P.O. Box 127 City				(715) 34° ZIP Code		Simple of I	Doing	· · · · · · · · · · · · · · · · · · ·	<del></del>	To Cine	<del>*************************************</del>	
ully Stevens Point	ı		1 )	ZIP Gode	J'	Signature or m	Person Doing	VVork	1	Date Sign	ĕd	



# **SMITHGROUP**

44 EAST MIFFLIN STREET SUITE 500 MADISON, WI 53703 608.251.1177 www.smithgroup.com

O f  $\Box$ o f Regents

Boar

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Revisions: lo. Date:

1 21AUG19 ADDENDUM 1

Graphic Scale AS SHOWN

K-19-001 Number Date 07/30/2019 Issued

CS100

INCLUDED FOR REFERENCE ONLY Number 

1. W.H.C. SPECIFICATIONS

Wherever W.H.C. appears in this specification it refers to the <u>Standard</u> Specification of Road and Bridge Construction, Edition 1963 of the State Highway Commission of Wisconsin. EXCEPTION: This contract shall be a lump sum contract and the method of measurement and the basis of unit price payment according to W.H.C. does not apply.

GENERAL CONSTRUCTION

2. EARTHWORK

Shall conform to W.H.C. Sections 201, 204, 205, 207, 208, 211 and 213. Contractor shall furnish all fill material required. All grades shown on plans are finished grades.

3. CHAIN LINK FENCE

Shall conform to W.H.C. Section 616.

A. Materials

(1) Type - Cyclone Safeguard, Century, or approved equal.

(2) Height - 4 ft. and 20 ft. as shown on the plans.

(3) Fabric - Hot-dip galvanized chain link 2 inch mesh, 9 guage, ASTM A392-55T, Class II installed with barbs down. aluminum coated. Class II chain link fabric conforming to ASTM A491-63T is acceptable. All fabric shall be placed on the diamond side of the fence posts.

(4) Top rail and intermediate rail - 1 5/8" O.D. pipe, 2.27 lbs./ft. or 1 5/8" x 1 1/4" roll form section.

(5) Line posts - 2 1/2" 0.D. pipe 3.65 lbs./ft. or Junior"H" column 2.72 lbs./ft. spaced at not more than 10° 0.C.

Posts for 20'-0" high fence shall be 4" O.D. pipe 9.1 lbs./ft.

(6) Gate posts - 3" O.D. pipe 5.79 lbs./ft.

(7) Terminal posts and corner posts - 3" O.D. pipe 5.79 lbs./ft. or 3 1/2" x 3 1/2" roll formed sections with integral fabric loops 5.14 lbs./ft.

(8) Tension wire - bottom tension wire shall be 7 gauge galvanized steel wire.

(9) Gate frames - 1 5/8" O.D. pipe 2.72 lbs./ft. Provide latch and center stop and keeper.

(10) Post settings - 4 ft. fence shall have posts set 3'-0" deep in concrete. Concrete shall be 4'-0" deep and 10" diameter. 20 ft. fence shall have posts set 4'-0" deep in concrete. Concrete shall be 5'-0" deep and 16" diameter.

65/68

EXISTING BASEBALL BACKSTOP FENCE SPECIFICATION INFORMATION

